Providence, Rhode Island

Proposed Commercial Redevelopment

July 2018
Revised December 2019
Revised August 2020
Revised November 2020

TRAFFIC IMPACT STUDY





Proposed Commercial Redevelopment Providence, Rhode Island

TRAFFIC IMPACT STUDY

Prepared by: BETA GROUP, Inc.

Prepared for: Mr. Michael Voccola

NMS Realty Trust

1140 Reservoir Avenue

Cranston, Rhode Island 02920

July 2018 Revised December 2019 Revised August 2020 Revised November 2020





July 25, 2018 Revised December 6, 2019 Revised August 21, 2020 Revised November 4, 2020

Mr. Michael Voccola NMS Realty Trust 1140 Reservoir Avenue Cranston, RI 02920

Re: Proposed Commercial Redevelopment

Dean Street and Kinsley Avenue

Providence, Rhode Island

Dear Mr. Voccola:

BETA Group, Inc., has completed an update to our revised August 2020 Traffic Impact Study in order to address changes made to the site redevelopment proposal that includes several commercial uses in the City of Providence, Rhode Island. The project is located on the westerly side of Dean Street between Kinsley Avenue and Harris Avenue. The parcel is defined by Assessor's Plat 26 Lot 234, AP 27 Lots 36 and 269, which together contain approximately four acres of land.

Based upon information provided by your office, and a review of the current site plan prepared by *DiPrete Engineering*, it is our understanding that the redevelopment project will include subdivision of the property into three commercial lots. Two of the lots will remain unchanged from the original proposal and include separate buildings containing a self-storage facility and small retail use. The third lot has been modified to eliminate the fast-food restaurant with a drive-through while maintaining the convenience market with gas station. Access and egress to the site will be provided from new driveways on Dean Street and Harris Avenue as originally proposed, and from two modified driveways on Kinsley Avenue with a common access across all parcels.

The study included herein, was conducted to determine the adequacy of the existing servicing roadways to accommodate anticipated traffic to be generated by the commercial redevelopment project. An analysis of potential impacts to the roadway capacity and safety has been completed and is discussed in the following report.

Very truly yours, BETA Group, Inc.

Paul J. Bannon

Senior Project Manager

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1.0 Introduction

The objective of the following study is to assess the potential traffic impacts associated with a proposed commercial redevelopment project in the City of Providence, Rhode Island. The subject property is situated on a parcel of land on the westerly side of Dean Street between Kinsley Avenue and Harris Avenue. Refer to the Figure 1, Project Vicinity Map, on the following page for the project location within the city.

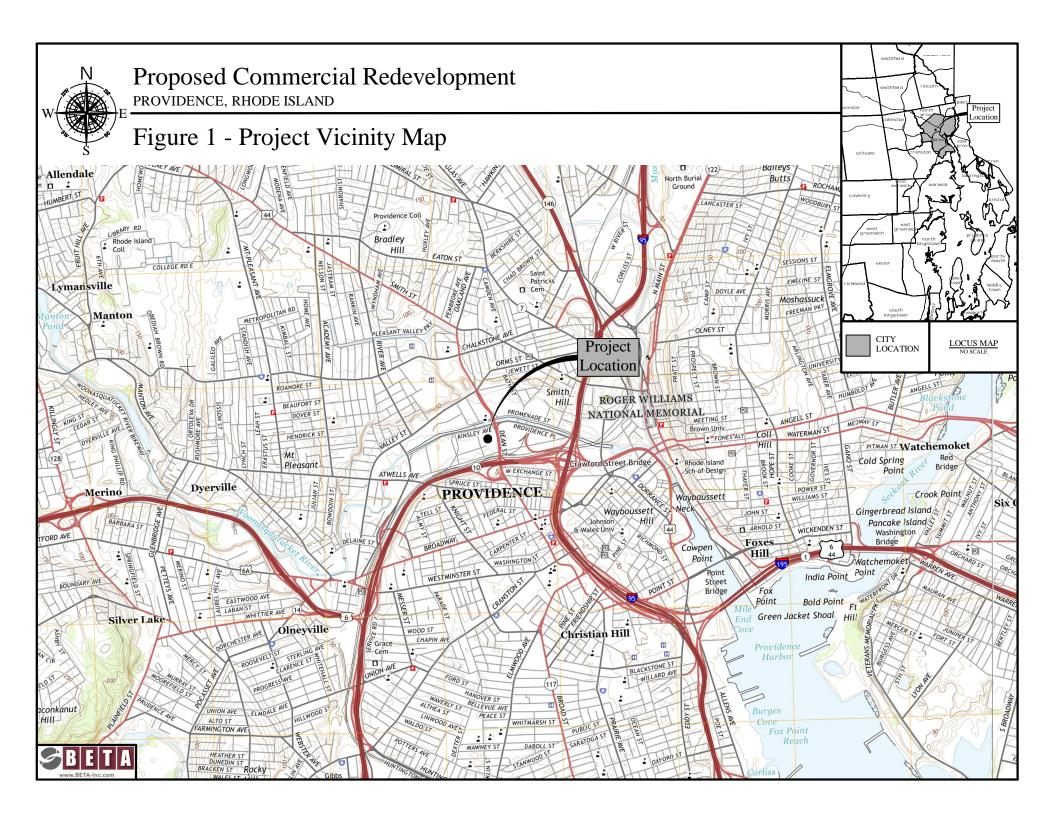
The development proposal consists of the construction of four separate buildings containing a 5-story, 805-unit self-storage facility, a 6,500 square foot retail store, and a 5,500 square foot convenience store/gasoline station with sixteen (16) vehicle fueling positions. Parking for the self-storage facility (25), retail building (23), and convenience store/gasoline station (25) including six electric charging station will be provided adjacent to each building, yielding a total of 79 parking spaces for the development. Access and egress will be provided from new driveways on both Dean Street and Harris Avenue, and two modified driveways on Kinsley Avenue. All proposed commercial uses will be interconnected via an internal driveway linking the parking areas and to all proposed points of access to the adjacent roadways.

The study summarized herein focused on both traffic flow efficiency and safety along Dean Street, Kinsley Avenue, and Harris Avenue in the immediate vicinity of the subject property, and at the proposed driveways. The impacts associated with the site related traffic have been defined and evaluated in accordance with standard traffic engineering guidelines and procedures.

The traffic engineering study completed for this project included the following:

- A traffic counting program to define the existing traffic patterns and operational characteristics
 along the servicing roadways including Dean Street, Kinsley Avenue, and Harris Avenue. The data
 collection included manual turning movement counts (TMCs) at the intersections of Dean Street
 with Kinsley Avenue/Providence Place and with Pleasant Valley Parkway/Promenade Street and
 at the intersection of Kinsley Avenue with Acorn Street. Automated traffic recorded (ATR) counts
 were collected on Kinsley Avenue and Harris Avenue.
- An inventory of the physical roadway characteristics of Dean Street, Kinsley Avenue, and Harris
 Avenue in the project area to determine the adequacy of the existing roadway geometric features
 in reference to safety and operations.
- An analysis of accident records obtained from the Providence Police Department to determine if
 there are any safety concerns relative to the frequency, severity, or pattern of crashes in the
 project area.
- An estimate of future traffic volumes for the proposed commercial development was calculated using data from the "Trip Generation" Manual, an informational report published by the Institute of Transportation Engineers (ITE).





- Evaluation and analysis of the traffic safety and operational issues for existing and future traffic conditions.
- Development of recommendations where necessary, that would be required to maintain safe and efficient traffic flow in the project area.

2.0 Project Area

As noted in the previous section, the subject property is situated on the westerly side of Dean Street between Harris Avenue to the south and Kinsley Avenue to the north. The property had previously contained a commercial building for *The Providence Journal* newspaper operation, which was razed several years ago, and has been for sale for commercial redevelopment for some time. Figure 2 on the following page depicts the general project area, and the boundary lines of the subject property.

Land use in the immediate area can be described as predominately commercial and industrial along Dean Street, Kinsley Avenue, and Harris Avenue. High density residential properties are situated to the east along Providence Place and Promenade Street. Immediately abutting the property to the west are industrial businesses including *Delta Auto Body* and *Capco Steel*. To the north, on the opposite side of Kinsley Avenue, is the *Woonasquatucket River*. To the east, on the opposite side of Dean Street, is *The Providence Journal*, a newspaper company. To the south, on the opposite side of Harris Avenue, are the *Amtrak* railway and Route 6/10 highway corridors. Further east along Providence Place is the *Providence Place Mall*.

Dean Street and Kinsley Avenue will serve as the primary access routes to the redeveloped property, with Harris Avenue providing a secondary access. Based upon the volume of traffic serviced along the immediate servicing roadways, and the minor volume of site-related traffic anticipated with the redevelopment project, a study impact area was defined for the project. The limits of our analysis focused on Dean Street, Kinsley Avenue, and Harris Avenue in the immediate vicinity of the property including the Dean Street intersection with Kinsley Avenue and the site driveway intersections.

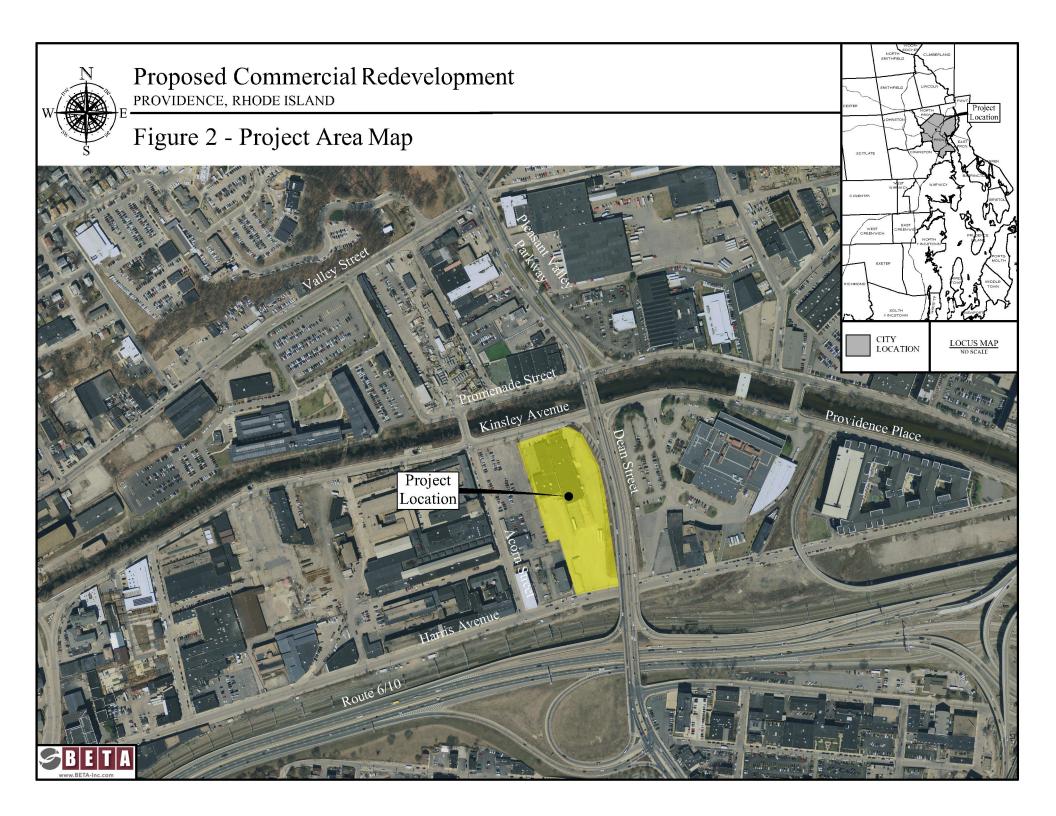
3.0 Existing Conditions

3.1 Roadways

Dean Street

Dean Street is a north/south urban minor arterial between Kinsley Avenue/Providence Place to the north and Westminster Street to the south. It provides immediate local access to abutting properties but also links to higher order facilities including the Route 6/10 interchange to the south. In the project area, Dean Street is approximately 57 feet wide consisting of two 12-foot travel lanes and two 1-foot shoulders in each direction separated by a 5-foot wide raised concrete median.





In addition, the I-95 northbound exit for Providence Place Mall, as part of the multi-exit ramp at Exit 22,

runs on the easterly side of Dean Street, which is separated by a concrete barrier. In the project area, the pavement surface can be classified as being in good condition as it was recently resurfaced as part of the Pleasant Valley Parkway bridge replacement project.

Concrete curbing with a mixture of concrete and bituminous sidewalks are provided on the westerly side of Dean Street. Cobra-head light fixtures are located along both sides of the corridor for nighttime



illumination. There is no posted speed limit in the project area and was assumed at 25 mph due to the urban nature of the area. The above photograph depicts the typical characteristics of Dean Street looking South with the subject property on the right.

Kinsley Avenue

Kinsley Avenue is an east/west urban major collector between Eagle Street to the west and Dean Street to the east. In the project area, Kinsley Avenue is approximately 34 feet wide consisting of one travel lane

in each direction and a bicycle lane in the eastbound direction. The pavement surface can be classified as being in good condition as it was recently resurfaced as part of the bridge project as previously described. Granite curbing and cement concrete sidewalks are provided on both sides of Kinsley Avenue. Ornamental lighting has been provided along both sides of the corridor for nighttime illumination. There is no posted speed limit along the road and therefore was assumed to be 25



mph due to the urban nature of the area. The adjacent photograph depicts the typical characteristics of Kinsley Avenue looking east with the subject property on the right.



Harris Avenue

Harris Avenue is an east/west urban major collector between Broadway to the southwest and Providence Place to the east. In the project area, Harris Avenue is approximately 40 feet wide consisting of a single

travel lane and a parking lane in each direction; however, no pavement markings are provided to delineate these lane uses. The pavement surface can be classified as being in fair condition with visible longitudinal cracking and patching. Granite curbing and cement concrete sidewalks are provided on both sides of Harris Avenue. Lighting on utility poles is located along the northerly side of the corridor. There is no posted speed limit in the project area and therefore was assumed to be 25



mph. The adjacent photograph depicts the typical characteristics of Harris Avenue looking east with the subject property on the left.

3.2 Intersections

<u>Dean Street/Pleasant Valley Parkway at Kinsley Avenue/Providence Place/Promenade Street</u>

Dean Street/Pleasant Valley Parkway intersect Kinsley Avenue/Providence Place and Promenade Street to form an unconventional signalized four-way junction with the Woonasquatucket River running through the middle of the intersection. Dean Street and Pleasant Valley Parkway combine to form the north/south roadway with the Woonasquatucket River Bridge being the northerly limit of Dean Street as the road extends to the north as Pleasant Valley Parkway. Promenade Street intersects with Pleasant Valley Parkway on the north side of the bridge and Kinsley Avenue/ Providence Place intersect Dean Street on the south side of the bridge.

The Dean Street northbound approach at Kinsley Avenue provides a separate left turn lane, a thru lane, and a shared thru/right turn lane. The Dean Street/Pleasant Valley Parkway southbound approach on the bridge provides a separate left turn lane, a thru lane, a shared thru/right turn lane, and a bike lane. The Kinsley Avenue eastbound approach provides a shared left turn/thru lane, a bike lane, and a channelized yield-controlled right turn lane. Providence Place is a one-way eastbound roadway.

The Pleasant Valley Parkway northbound approach at Promenade Street provides a left/thru lane and a thru lane while the southbound approach provides two thru lanes and a short channelized right turn lane. Promenade Street is a one-way westbound roadway running along the northern bank of the



Woonasquatucket River. The Promenade Street westbound approach provides a left turn lane, a shared left turn/thru lane, and a channelized yield-controlled right turn lane.

As previously mentioned, this intersection was recently reconstructed as part of the Pleasant Valley

Parkway bridge replacement project. The improvements made at the intersection in addition to the bridge included a new traffic signal system, roadway surface, pavement markings, signing, curbing, and sidewalks as depicted on the adjacent photograph looking east along Kinsley Avenue.

The intersection was determined to operate in a fully actuated mode utilizing a single controller operating in a three-phase manner to service the offset minor



approaches. Phase 1 services the Dean Street and Pleasant Valley Parkway northbound/southbound through/right concurrent movements. Phase 2 services the Pleasant Valley Parkway northbound at the Promenade Street junction, and the Kinsley Avenue eastbound concurrent movements. Phase 3 services the Pleasant Valley Parkway southbound at the Kinsley Avenue junction, and the Promenade Street westbound concurrent movements. In addition, pedestrian accommodations are present at the intersection including curb ramps, marked pedestrian crosswalks, pedestrian signal heads with push buttons on all legs of the intersection, except for the north and south legs between both junctions.

Kinsley Avenue at Acorn Street

Kinsley Avenue intersects Acorn Street to form a four-way junction under all-way *Stop* control. All approaches to the intersection provide a single multipurpose lane. Granite curbing and cement concrete sidewalks are provided at the intersection including curb ramps. Marked pedestrian crosswalks are provided across all legs of the intersection except for the westbound approach. Lighting is provided on ornamental light



poles for nighttime illumination of the intersection.



3.3 Traffic Flow Data

Existing traffic flow characteristics for this area were developed from a traffic counting program conducted by BETA including Manual Turning Movement Counts (TMC) and Automatic Traffic Recorder (ATR) counts in March and April 2018. Manual turning movement counts were conducted at the intersections of Pleasant Valley Parkway with Promenade Street and Dean Street with Providence Place/Kinsley Avenue, and at the intersection of Kinsley Avenue with Acorn Street. Data was collected on a weekday between 7:00 and 9:00 AM and 4:00 to 6:00 PM when the site and surrounding roadway service their highest daily traffic volumes.

The automatic traffic recorder counts (ATR) were conducted on Kinsley Avenue, between Acorn Street and Dean Street and on Harris Avenue, just west of Acorn Street. Based upon the ATR data obtained, Kinsley Avenue was found to service an Average Daily Traffic (ADT) volume of approximately 5,900 vehicles. On a typical weekday along Kinsley Avenue, traffic volumes begin to increase at 5:00 AM, with the morning commuter peak hour occurring between 8:00 and 9:00 AM. During this hour, an average of 590 vehicles was recorded. After 9:00 AM, volumes decrease to between 280 and 400 vehicles per hour until the late afternoon peak of 530 vehicles serviced between 4:00 and 5:00 PM.

Harris Avenue in the project area was found to service an Average Daily Traffic (ADT) volume of approximately 4,500 vehicles. On a typical weekday along Harris Avenue, traffic volumes begin to increase at 6:00 AM, with the morning commuter peak hour occurring between 8:00 and 9:00 AM. During this hour, an average of 330 vehicles was recorded. After 9:00 AM, volumes decrease to between 200 and 260 vehicles per hour until the late afternoon peak of 430 vehicles serviced between 5:00 and 6:00 PM.

The turning movement count data found that Dean Street services approximately 2,810 vehicles during the weekday morning peak hour between 8:00 and 9:00 AM with approximately 1,310 vehicles northbound and 1,500 vehicles southbound. During the same time period, Kinsley Avenue serviced approximately 630 vehicles with approximately 520 vehicles eastbound and 110 vehicles westbound. During the weekday afternoon peak hour between 4:30 and 5:30 PM, Dean Street serviced 3,300 vehicles with approximately 1,540 vehicles northbound and 1,760 vehicles southbound. During the same time period, Kinsley Avenue serviced 525 vehicles with approximately 450 vehicles eastbound and 75 vehicles westbound. Figure 3 on the following page depicts the daily peak hour turning movement volumes at the study intersection. Complete count information can be found in the Appendix.

4.0 SAFETY ANALYSIS

To determine if there are any limiting factors affecting safety relating to access to the proposed commercial project, the physical characteristics of Dean Street, Kinsley Avenue, and Harris Avenue in the project area were investigated. These limiting factors would potentially include horizontal or vertical alignment changes or roadside obstructions that limit sight distances for vehicles traveling along the road or entering the road from a side street or driveway location. In this instance, the sight distance standard is necessary to permit turning vehicles to safely enter and exit the site driveways.

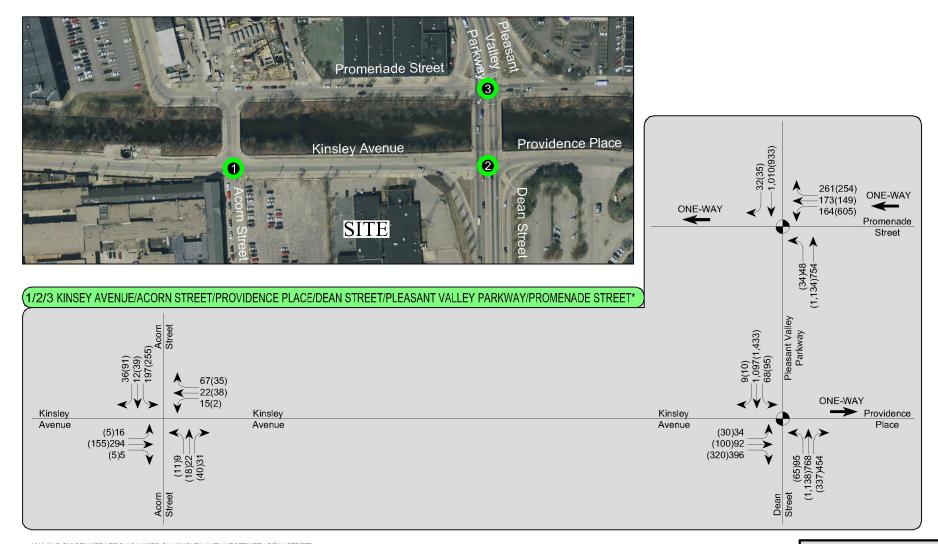




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Figure 3 - Existing Traffic Volumes



*AM AND PM PEAK TRAFFIC VOLUMES ON KINSLEY AVENUE BETWEEN DEAN STREET AND ACORN STREET WERE BALANCED BASED ON AUTOMATIC TRAFFIC RECORDER (ATR) DATA COUNTS.



LEGEND:

TURN MOVEMENT

XXX AM PEAK VOLUMES (8:00 TO 9:00)
(XXX) PM PEAK VOLUMES (4:30 TO 5:30)

STUDY INTERSECTION

TRAFFIC SIGNAL

The horizontal alignment of Dean Street in the project area can be described as relatively straight with a gradual curve along the subject property frontage. The vertical alignment can be described as generally level north of Kinsley Avenue across the Woonasquatucket River Bridge and with a gradual incline along the site property frontage heading south from Kinsley Avenue to a crest vertical curve at the Route 6/10 bridge overpass. Based upon the existing roadway geometry as described, the available sight distance at the proposed right in/out site driveway location on Dean Street was determined to be greater than 500 feet through the signalized junction with Promenade Street to the north. This value is greater than AASHTO's recommended minimum sight distance of 155 feet based on the prima facie speed limit of 25 mph and is sufficient for speeds in excess of 40 mph. It should be noted that speeds are highly variable due to the signal-controlled Kinsley Avenue and Promenade Street junctions, where vehicles are turning onto Dean Street at a low speed or accelerating from a stop condition.

Reviewing the existing right turn movement from Kinsley Avenue, the available sight distance from the yield line on the channelized right turn ramp at Dean Street was determined to be greater than 350 feet through the signalized junction with Promenade Street to the north. As previously noted, the high type ramp design for the right turn intersects Dean Street at skewed angle, hindering sight lines for drivers

turning onto Dean Street from Kinsley Avenue. This angle forces a driver to either use their driver's side mirror while encroaching into the southbound travel lane for proper positioning, or physically turning their bodies to see the necessary distance to the north to determine if vehicles approaching. This requirement can be difficult for elderly or physically impaired drivers to accomplish sufficiently to permit safe movements or efficient movements onto Dean Street.



This condition could result in drivers being overcautious and not taking advantage of available gaps, or not yielding properly to oncoming traffic which was observed. The intersecting angle of the ramp is shown in the adjacent photograph.

The horizontal and vertical alignment of Kinsley Avenue in the project area can be described as generally straight and level, allowing sight distances in excess of 500 feet through the all-way *Stop* controlled intersection with Acorn Street to the west of both driveways and 250 feet through the signalized junction with Dean Street to the east from the proposed eastern driveway. The defined roadway geometry results in values that are greater than the 155-foot minimum stopping sight distance required according to AASTHO criteria for the prima facie speed limit of 25 mph. It is important to note that speeds are highly



variable due to the controlled junctions at Dean Street and Acorn Street, where vehicles are turning onto Kinsley Avenue at a low speed or slowing to the stop line at the controlled junctions.

The horizontal and vertical alignment of Harris Avenue in the project area can be described as generally straight and level, allowing sight distances at Harris Avenue in excess of 500 feet to the east and west of the proposed site access driveway intersection. The defined roadway geometry results in values that are greater than the 155-foot minimum stopping sight distance required according to AASTHO criteria for the prima facie speed limit of 25 mph, and the 360 feet based on the observed speeds between 40 and 45 mph. It should be noted that parking on the northerly side of Harris Avenue may create an obstruction that limits sight distance for vehicles exiting the proposed site driveway onto Harris Avenue if restrictions were not implemented adjacent to the new driveway. In an effort to enhance safety and limit vehicles parking in the vicinity of the proposed site driveway intersection, in accordance with the City of Providence regulations, parking restriction signs should be placed a minimum of 25 feet east and west of the proposed site driveway prohibiting parking on the northerly side of Harris Avenue along the property frontage.

Also, as part of our analysis, a review of accident statistics was completed. Data was reviewed from the City of Providence Police Department at the intersection of Dean Street with Kinsley Avenue with emphasis on the Kinsley Avenue channelized right turn ramp to determine if there were any safety concerns specific to this movement. Data was obtained for a two-year period as the bridge was under construction in 2015 and 2016 with different traffic patterns than current conditions. A review of the information at the study intersection found that a total of 33 crashes (avg. 17 per year) occurred over the study period (2017-2018), with 12 involving an injury. Summarizing the data, eight of the total crashes (approximately 25%) occurred at the Kinsley Avenue channelized right turn lane with five rear end crashes, two side-swipe collisions, and an angle type collision. The rear end accidents at this approach can be attributed to the skewed angle that limits sight lines for motorists yielding to Dean Street southbound through traffic, coupled with the wide turn ramp style design that allows for higher approach speeds. A summary of the crash data is provided in the Attachments for reference.

A review of the historical accident data obtained from the local police, combined with a field review of the intersection operations, and review of existing roadway geometry and physical features, determined that traffic related safety enhancements could be considered at this intersection to improve overall operations and safety. As noted, the sight line visibility and higher approach speed concerns identified at the Kinsley Avenue channelized right turn approach could be mitigated with a reconfiguration of the right turn ramp to be more consistent with an urban low speed environment. This is reconfiguration is appropriate as the roadways and intersection accommodate pedestrians and separate bike lanes along Kinsley Avenue and Dean Street through the signalized intersection.

Based upon these considerations where vehicular, pedestrian and bicycle safety can be enhanced through minor geometric changes, it is recommended that the Kinsley Avenue channelized right turn be modified with a smaller turning radius to enhance sight lines while mitigating approach speeds and improving pedestrian and bicycle interactions. The control of the right turn should be maintained as



yield conditions separate from signal control in order to provide the most efficient operations for the movement. It should also be noted that in tightening up the intersection, the design will also increase the distance between the driveway and the adjacent intersection to approximately 200 feet. This design creates ample separation between the Kinsley Avenue channelized right turn and the proposed site driveway on Dean Street. In addition, to restrict left turn entering the site at the proposed restricted right in/out driveway on Dean Street, it is recommended to extend the existing northbound left turn restriction on Dean Street be extended by approximately 140 feet northerly using pavement markings and vertical devices (flexible delineators or similar) across from the proposed right turn in/out driveway on Dean Street.

It should also be noted that this design recommendation would be a permanent condition based upon recently developed plans by the City of Providence for the *Woonasquatucket River Greenway* project that proposes to modify Kinsley Avenue to include a dedicated bike lane that will require the road to be converted to one-way eastbound along the subject property frontage including maintaining the existing geometry of the right turn from Kinsley Avenue and eliminating the Dean Street northbound left turn traffic at the intersection with Promenade Street.

This change will eliminate the ability of northbound traffic on Dean Street to access the site forcing vehicles to go through two traffic signals and loop back to the south using Promenade Street and Acorn Street. In order to mitigate this adverse impact, a long-term alternative plan has been developed to allow direct left turns from Dean Street within the existing left turn lane for the traffic signal at Kinsley Avenue. This option will greatly improve operations in this area by allowing a movement that would exist regardless under the original design and reduce the resultant additional vehicle and pedestrian conflicts created by the left turn restrictions caused by changing Kinsley Avenue to one-way eastbound. In addition, under the City project, the Dean Street northbound left turn traffic at the intersection with Promenade Street will be eliminated forcing vehicles to turn right at the intersection with Kinsley Avenue/Providence Place and loop to the west using Providence Place and Bath Street.

5.0 IMPACT ANALYSIS

5.1 Trip Generation

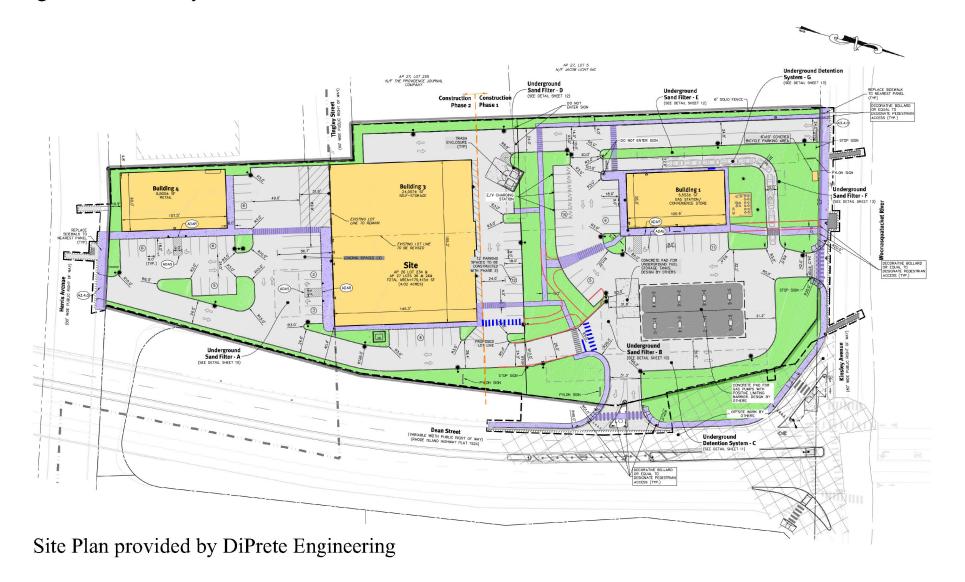
To determine the traffic impact of a proposed development, estimates of anticipated traffic to be generated by a particular land use must be calculated. As previously discussed, the redevelopment proposal consists of the construction of a 5-story, 805-unit self-storage building, a 6,500 square foot building for retail use, and a 5,500 square foot convenience store/gasoline station with 16 vehicle fueling positions. Access and egress are proposed from new driveways on Dean Street (restricted to right in/out), and Harris Avenue, and two modified driveways on Kinsley Avenue. Figure 4 on the following page depicts the site layout and access plan provided by *DiPrete Engineering*.



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Figure 4 - Site Layout





For this site, projected traffic volumes for the commercial project were based on the use of trip generation factors. These factors are taken from the "Trip Generation" manual, an informational report published by the Institute of Transportation Engineers (ITE), a national professional organization for traffic and transportation engineers. The data provided in the ITE report are based on extensive traffic studies for various types of land uses (residential, commercial, industrial, etc.). This data has been found to be very reliable and provides a sound basis for estimating future trips to new development projects. For the proposed commercial project, Land Use Code 151 Mini-Warehouse, Land Use Code 820 Shopping Center, and Land Use Code 960 Super Convenience Market/Gas Station were reviewed for applicability in developing an estimate of site related vehicles trips.

The appropriate worksheets from the manual are included in the Appendix, along with the trip estimate calculations. Table 1 below summarizes the estimated trip volumes calculated for this project for the morning and afternoon peak traffic conditions.

TABLE 1 – Trip Generation Estimate

	Description	Enter	Exit	Total
AM Peak Hour				
ITE Land Use Code 151	Mini-Warehouse	6	5	11
ITE Land Use Code 820	Shopping Center	4	2	6
ITE Land Use Code 960	Super Convenience Market/Gas Station	225	225	450
	Total	235	232	467
<u>PM Peak Hour</u>				
ITE Land Use Code 151	Mini-Warehouse	8	8	16
ITE Land Use Code 820	Shopping Center	12	13	25
ITE Land Use Code 960	Super Convenience Market/Gas Station	<u>185</u>	184	368
	Total	205	205	410

It is important to note that the compatibility of uses, where a single site trip is generated for the multiple uses being proposed, is referred to as "internal-capture" where a driver would potentially visit two or more of the proposed uses within the proposed development. Consequently, these internal trips capture would allow reduction of the total trips generated by a multiple use development. In addition, to the internal capture potential, it is estimated that between 40% and 60% of trips generated by the proposed convenience store/gasoline station will not be new to the servicing roadways. The ITE manual provides information on what is referred to as "pass-by" trips, or those trips associated with the site that are already on the servicing roadways and turn into and out of a business and continue to their destination. Therefore, these pass-by vehicles would not be "added" to the adjacent servicing roadway but would be



diverted vehicles in to and out of the new development. However, to be conservative, no reduction for pass-by or internal-capture trips were considered in our analysis.

5.2 FUTURE TRAFFIC VOLUMES

In order to properly assess the impacts of a development, future traffic conditions of area roadways should be estimated for the period when the development is constructed and fully occupied. Typically, the expansion of base traffic is calculated when a project is to be constructed over an extended period (+3 to 5 years). In all instances, area growth that may affect capacity results should be considered. The traffic growth estimate was based on comparison of current traffic volumes to historical traffic volumes in the project area, which has seen a minor increase, a review of recently approved projects by the City in the immediate area, and available statewide planning growth data.

For this project, a conservative annual growth rate of 1.0 percent was utilized. This rate was applied to the existing volumes to establish a future 2023 No-Build traffic condition on the servicing roadways. The proposed commercial project was then added to the No-Build condition to establish the future 2023 Build traffic condition. In addition, in coordination with the City, as noted earlier it was determined that the *Woonasquatucket River Greenway*, an urban trail, is proposed along the Kinsley Avenue corridor extending east through Providence Place to the downtown. The project proposes conversion of the current two-way traffic on Kinsley Avenue to one-way eastbound to accommodate the addition of a separated urban trail. In order to determine the potential impact of the proposed extension project to the site within the project area, the proposed alteration to the traffic circulation was analyzed as an alternative. Figure 5 on the following page depicts the estimated future build traffic volumes at the study intersections. Site distribution figures are also provided in the Appendix.

In developing the intersection volumes to be analyzed under build conditions, a directional distribution of the site traffic was estimated. The distribution was based on current traffic patterns along Dean Street and Kinsley Avenue. For the self-storage and retail components of the proposed development, it is estimated that 40% of the site traffic will arrive from and depart to the north, 20% will arrive from and depart to the south, 10% will arrive from and depart to the east, and 30% will arrive from and depart to the west during both the morning and afternoon peak hours. For the convenience store/gasoline station and fast-food restaurant with drive-through window components of the proposed development, it is estimated that 40% of the site traffic will arrive from the north and depart to the south, 20% will arrive from the south and depart to the north, 10% will arrive from the east and depart to the west, and 30% will arrive from the west and depart to the east during both the morning and afternoon peak hours.

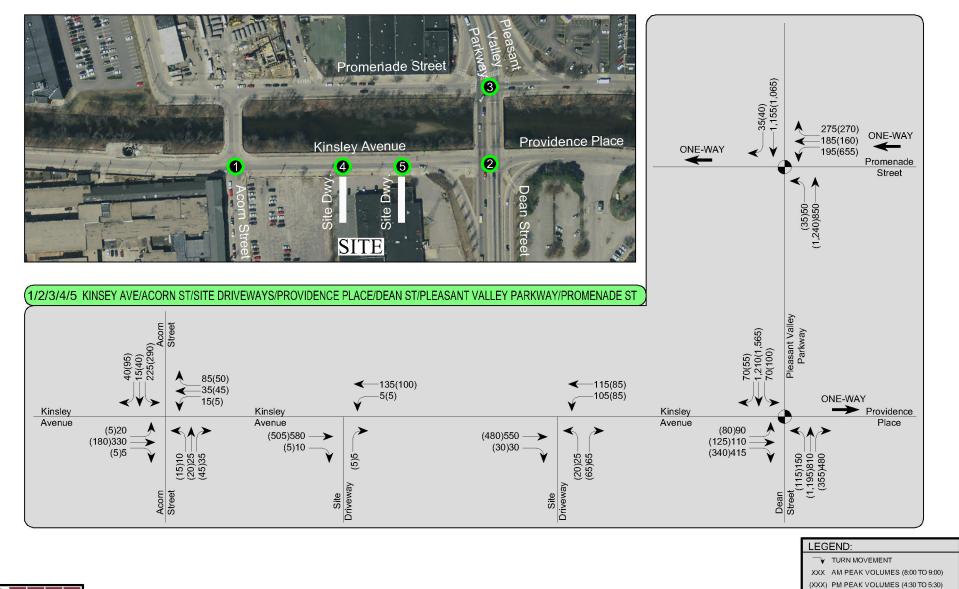




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Figure 5 - Future Traffic Volumes



STUDY INTERSECTION
 TRAFFIC SIGNAL



5.3 OPERATIONAL ANALYSIS

The key to any traffic impact analysis is the evaluation of roadway operations during peak traffic periods on the servicing roadway system. This situation would occur when the site-generated traffic, combined with the traffic volumes on the main roadway, result in the highest one-hour volume serviced along a roadway segment, or through an intersection. Review of record traffic data found that the weekday AM and PM peak hours would represent this worst-case combination of site-generated traffic with the servicing roadway peak traffic period.

The 2010 Highway Capacity Manual methodology provides the most accurate means of evaluating traffic capacity and delays for roadways and intersections. The results of this procedure are expressed in terms of Level of Service (LOS). Level of Service is a qualitative measure of traffic flow efficiency based on anticipated vehicle delays. For example, LOS "A" represents the best condition with little or no delay, while LOS "F" indicates that the roadway/intersection is at full capacity resulting in extended vehicle delays and potential queuing. Table 2 outlines the Level of Service delay criteria presented in the Highway Capacity Manual for signalized and unsignalized intersections.

TABLE 2 – Highway Capacity Manual Criteria

	Unsignalized Delay	Signalized Delay
Level of Service	Per Vehicle (sec)	Per Vehicle (sec)
Α	<10	<10
В	>10 and <15	>10 and <20
С	>15 and <25	>20 and <35
D	>25 and <35	>35 and <55
E	>35 and <50	>55 and <80
F	>50	>80

The Pleasant Valley Parkway intersection with Promenade Street and the Kinsley Avenue intersections with Dean Street, Acorn Street, and the site driveways were all analyzed for the weekday morning and afternoon peak hours. The capacity analysis worksheets are included in the Appendix and Tables 3 through 6 on the following pages summarize the results of the analyses.

Table 3 on the following page depicts the current conditions at the study intersections. As can be seen in the table, the signalized intersection of Dean Street/Pleasant Valley Parkway with Kinsley Avenue and Providence Place/Promenade Street operates at acceptable Level of Service D or better during both the AM and PM peak hours. Two movements during the AM peak hour and three movements during the PM peak hour were found to operate with greater delays due to the high peaking volumes during these periods. The Dean Street northbound left turn movement operates currently under permitted conditions resulting in delays in excess of 80 seconds, though due to the low turning volume, queueing is not extensive and typically the movement is serviced within the permitted phase and change interval. During the morning peak a maximum queue of seven vehicles was observed, with an average of three vehicles, which is consistent with the analysis.



TABLE 3 – Level of Service Summary (Existing Conditions)

			2018 E	XISTING	COND	ITIONS		
		AM	Peak Hour			PM	Peak Hour	
Location / Movement			95 th %				95 th %	
	LOS	Delay	Queue	v/c	LOS	Delay	Queue	v/c
			Length (veh.)				Length (veh.)	
Dean Street at Kinsley Avenue.	/Provid	dence Pla	ace (S) ¹					
Dean St. NB Left	F	157.0	7	1.10	F	81.4	5	0.78
Dean St. NB Thru/Right	В	19.8	15	0.75	Ε	79.9	27	1.09
Dean St. SB Left	Α	6.9	1	0.13	Α	4.7	1	0.15
Dean St. SB Thru/Right	Α	1.4	0	0.42	Α	1.4	0	0.54
Kinsley Ave. EB Left/Thru	D	49.2	6	0.60	D	54.1	6	0.63
Kinsley Ave. EB Right	F	153.3	16	1.24	F	177.7	15	1.28
OVERALL	D	36.2	-	-	D	54.0	-	-
Pleasant Valley Parkway at Pro	omena	de Stree	et (S) ¹					
Pleasant Valley Pkwy. NB	Α	2.0	1	0.44	Α	3.3	1	0.69
Pleasant Valley Pkwy. SB Thru	В	19.5	12	0.62	С	25.1	12	0.67
Pleasant Valley Pkwy. SB Right	Α	0.1	1	0.04	Α	0.1	1	0.05
Promenade St. WB Left	С	30.0	5	0.38	D	35.7	12	0.73
Promenade St. WB Left/Thru	С	31.5	6	0.46	С	34.8	12	0.72
Promenade St. WB Right	В	13.4	4	0.52	В	18.7	6	0.47
OVERALL	В	14.5	-	-	В	18.8	-	-
Kinsley Avenue at Acorn Street	t (U)							
Kinsley Ave. EB	В	12.4	3	0.48	Α	10.0	1	0.26
Kinsley Ave. WB	Α	8.9	1	0.16	Α	8.8	1	0.12
Acorn St. NB	Α	8.9	1	0.10	Α	8.4	1	0.10
Acorn St. SB	В	11.7	2	0.39	В	13.1	4	0.54
OVERALL	В	11.4	-	-	В	11.4	-	-

⁽S) – Signalized

During the afternoon peak hour, the Dean Street northbound left lane, and thru/right lanes, and the Kinsley Avenue eastbound left/thru lane movements operate with greater delays at LOS F, E and LOS D where a maximum queue of 6, 27, and 6 vehicles were observed, respectively, which is consistent with the analysis. This queueing extends approximately 150, 675, and 150 feet, to the south and west respectively, during the busiest period of the day. The unsignalized Kinsley Avenue intersection with Acorn Street which is an *All-Way* Stop, operates efficiently with the critical movements experiencing minor delays of fewer than 14 seconds, representing LOS B or better during the daily peak hours of traffic.



⁽U) – Unsignalized

¹ Operates as a single intersection with one controller

TABLE 4 – Level of Service Summary (Future No-Build Conditions)

			2023 N	O-BUILE	COND	ITIONS		
		AM	Peak Hour			PM	Peak Hour	
Location / Movement			95 th %				95 th %	
	LOS	Delay	Queue	v/c	LOS	Delay	Queue	v/c
			Length (veh.)				Length (veh.)	
Dean Street at Kinsley Avenue	e/Provid	dence Pla	ace (S) ^{1,2}					
Dean St. NB Left	F	199.7	5	1.23	F	94.9	5	0.86
Dean St. NB Thru/Right	С	21.8	15	0.80	E	55.3	26	1.03
Dean St. SB Left	В	10.5	1	0.18	Α	7.8	1	0.20
Dean St. SB Thru/Right	Α	1.8	1	0.49	Α	1.6	1	0.60
Kinsley Ave. EB Left/Thru	С	34.3	5	0.36	D	40.9	6	0.48
Kinsley Ave. EB Right	F	87.3	17	1.05	F	111.3	15	1.11
OVERALL	С	29.1	-	-	D	37.7	-	-
Pleasant Valley Parkway at Pi	romena	de Stree	et (S) ^{1,2}					
Pleasant Valley Pkwy. NB	Α	1.5	1	0.41	Α	2.0	1	0.61
Pleasant Valley Pkwy. SB Thru	С	20.9	13	0.66	С	21.5	12	0.63
Pleasant Valley Pkwy. SB Right	Α	0.1	1	0.05	Α	0.7	1	0.05
Promenade St. WB Left	D	42.8	6	0.55	F	94.1	18	1.05
Promenade St. WB Left/Thru	D	49.3	9	0.70	F	90.3	18	1.03
Promenade St. WB Right	В	15.0	4	0.62	С	29.0	8	0.64
OVERALL	В	17.3	-	-	С	31.8	-	-
Kinsley Avenue at Acorn Stree	et (U)							
Kinsley Ave. EB	В	13.4	3	0.51	В	10.4	1	0.28
Kinsley Ave. WB	Α	9.2	1	0.17	Α	9.0	1	0.13
Acorn St. NB	Α	9.2	1	0.12	Α	8.6	1	0.12
Acorn St. SB	В	12.3	2	0.42	В	13.9	4	0.57
OVERALL	В	11.9	-	-	В	12.0	-	-

⁽S) – Signalized

Table 4 above represents the future design period taking into considerations base traffic growth as noted earlier along the servicing roadways. The subject development is not included in this "No-Build" analysis scenario. As can be seen, the signalized intersection of Dean Street/Pleasant Valley Parkway with Kinsley Avenue/Providence Place/Promenade Street continues to operate overall at an acceptable LOS D or better during the morning and afternoon peak traffic conditions; however, with the additional traffic associated with background growth in this area, the critical movements experiences greater delays, though some movements improve through optimization of the phase timing. The unsignalized Kinsley Avenue intersection with Acorn Street will continue to operate efficiently with the critical movements



⁽U) - Unsignalized

¹ Operates as a single intersection with one controller

² Optimized Timings

experiencing minor delays of fewer than 14 seconds, representing LOS B or better during the daily peak hours of traffic.

Table 5 on the following page presents the future conditions where the analysis found that the signalized intersection of Dean Street/Pleasant Valley Parkway with Kinsley Avenue/Providence Place/Promenade Street will operate overall in an acceptable manner at LOD D or better during the daily peak hours of traffic. All critical movements will operate at LOS D or better except for the Dean Street northbound left during the morning and afternoon peak periods, the Kinsley Avenue eastbound right during the morning peak period, and the Promenade Street westbound left and thru movements during the afternoon peak period. These critical movements will continue to operate at LOS F during the peak periods stated. The unsignalized Kinsley Avenue intersection with Acorn Street will operate efficiently with the critical movements experiencing minor delays of fewer than 16 seconds, representing LOS C or better during the daily peak hours of traffic.

Much of the delay increase during the peak periods is a direct result of background growth that may not be realized in the foreseeable future. Therefore, it is recommended that consideration of timing and/or phasing adjustments be coordinated with the RIDOT as traffic volumes increase in the immediate future to improve the efficiency of the intersection during the daily peak traffic conditions. It should be noted that this intersection was recently upgraded and designed to accommodate an additional signal head to control the northbound left turn lane which is currently operating in a permitted manner with no separate head. If at a future date this northbound left turn volume realizes greater delays and queueing resulting in an overflow of the dedicated turn bay, the new head can be installed to allow a protected/permitted left turn movement that would reduce delays and improve overall operations at the junction. This recommendation does not consider the change in Kinsley Avenue to one-way eastbound which would eliminate this movement at the signal and would be considered if necessary, only if the City project does not move forward.

In addition, as previously mentioned, it was determined that the *Woonasquatucket River Greenway*, an *urban trail*, is proposed along the Kinsley Avenue corridor through Providence Place and is scheduled to be constructed in the next two years. The project proposes conversion of the current two-way traffic on Kinsley Avenue to one-way eastbound to accommodate the addition of a separate urban trail on Kinsley Avenue as part of the extension of the *Woonasquatucket River Greenway*. Under this project, the Dean Street northbound left turn movement at the signalized intersection with Kinsley Avenue will be eliminated and redirected to the signalized intersection of Pleasant Valley Parkway with Promenade Street to the north. To address this change that would adversely impact the adjacent signalized intersection and create additional pedestrian and vehicle conflicts, the proposed site driveway on Dean Street is recommended to allow full entry access (change to allow left turns in that would currently use Kinsley Avenue), but remain restricted to right out only exiting vehicles. Converting Kinsley Street to one-way eastbound will also restrict the driveways on Kinsley Avenue to right in/out only movements as a result of the new traffic pattern proposed under the urban trail project. In order to determine the potential impact of the change to the site access within the project area, the proposed alteration to the traffic circulation was analyzed as a separate build "alternative" defined in Table 6.



TABLE 5 – Level of Service Summary (Future Build Conditions)

		2023	BUILD	CONDIT	TONS							
	AM	Peak Hour			PM	Peak Hour						
		95 th %				95 th %						
LOS	Delay	Queue	v/c	LOS	Delay	Queue	v/c					
		Length (veh.)				Length (veh.)						
/Provid	dence Pla	ace (S) ^{1,2}										
F	*	9	1.85	F	*	6	1.40					
С	21.8	15	0.80	E	68.7	27	1.07					
Α	9.6	1	0.18	Α	8.9	1	0.25					
Α	1.9	1	0.54	Α	2.8	1	0.71					
D	39.2	7	0.56	С	35.2	7	0.51					
F	*	17	1.08	D	43.9	12	0.84					
D	43.1	-	-	D	42.0	-	-					
Pleasant Valley Parkway at Promenade Street (S) ^{1,2}												
Α	1.6	1	0.44	Α	2.3	1	0.58					
С	22.0	14	0.71	С	24.3	14	0.71					
Α	0.1	1	0.05	Α	0.7	1	0.06					
D	47.5	8	0.65	F	*	21	1.39					
D	50.5	9	0.72	F	*	21	1.37					
В	18.8	5	0.65	D	41.1	9	0.78					
В	18.6	-	-	Е	64.3	-	-					
t (U)												
В	14.9	4	0.57	В	11.1	2	0.32					
Α	9.8	1	0.22	Α	9.5	1	0.16					
Α	9.5	1	0.12	Α	9.0	1	0.13					
В	13.5	3	0.47	С	16.2	5	0.63					
В	13.2	-	-	В	13.4	-	-					
e Drive	way (U)											
Α	8.8	0	0.01	Α	8.5	0	0.01					
В	12.6	0	0.01	В	11.8	0	0.01					
Drivev	vay (U)											
Α	9.3	1	0.12	Α	8.9	1	0.09					
С	17.6	1	0.26	В	14.7	1	0.20					
	/Provider F	LOS Delay /Providence Plane F	AM Peak Hour Post	AM Peak Hour Cos Delay Queue Cength (veh.)	AM Peak Hour Spth % Queue Length (veh.) V/c LOS	LOS Delay 95th % Queue Length (veh.) v/c LOS Delay /Providence Place (S) 1.2 F * 9 1.85 F * F * 9 1.85 F * C 21.8 15 0.80 E 68.7 A 9.6 1 0.18 A 8.9 A 1.9 1 0.54 A 2.8 D 39.2 7 0.56 C 35.2 F * 17 1.08 D 43.9 D 43.1 - - D 42.0 omenade Street (S) 1.2 S S A 0.7 C 24.3 A 1.6 1 0.44 A 2.3 C 22.0 14 0.71 C 24.3 A 0.1 1 0.05 A 0.7 * * * B 1.1 0.05 A	AM Peak Hour PM Peak Hour PSth % Queue Length (veh.) V/c LOS Delay Queue Length (veh.) Providence Place (S) 1.2 F * 9 1.85 F * 6 68.7 27					



⁽S) – Signalized (U) – Unsignalized

¹ Operates as a single junction with one controller

² Optimized Timings

^{*} Delay greater than 80 seconds.

TABLE 6 – Level of Service Summary (Future Build Conditions - Alternative)

			2023 BUILD (CONDIT	IONS (A	LTERNA	TIVE)			
		AM	Peak Hour			PM	Peak Hour			
Location / Movement			95 th %				95 th %			
	LOS	Delay	Queue	v/c	LOS	Delay	Queue	v/c		
			Length (veh.)				Length (veh.)			
Dean Street at Kinsley Avenue.					1					
Dean St. NB Thru/Right	D	49.5	21	1.01	F	*	28	1.10		
Dean Street SB Left	Α	8.1	1	0.15	Α	6.9	1	0.19		
Dean Street SB Thru	Α	3.2	2	0.57	Α	1.9	1	0.62		
Kinsley Ave. EB Left/Thru	С	32.9	7	0.46	Ε	59.5	9	0.79		
Kinsley Ave. EB Right	Ε	62.1	16	0.96	F	*	15	1.17		
OVERALL	С	31.8	-	-	D	49.9	-	-		
Pleasant Valley Parkway at Promenade Street (S) 1,2										
Pleasant Valley Pkwy. NB	Α	3.1	1	0.35	Α	8.1	1	0.55		
Pleasant Valley Pkwy. SB Thru	С	34.1	17	0.87	С	23.6	13	0.70		
Pleasant Valley Pkwy. SB Right	Α	1.4	1	0.07	Α	1.6	1	0.07		
Promenade St. WB Left	D	37.5	7	0.52	F	*	21	1.10		
Promenade St. WB Left/Thru	E	77.5	16	0.97	F	*	21	1.09		
Promenade St. WB Right	В	14.5	5	0.57	С	26.1	7	0.60		
OVERALL	С	28.2	-	-	D	39.3	-	-		
Kinsley Avenue at Acorn Street	t (U)									
Kinsley Ave. EB	В	13.7	4	0.54	В	10.7	2	0.31		
Acorn St. NB	Α	9.1	1	0.13	Α	8.6	1	0.14		
Acorn St. SB	В	12.9	3	0.49	С	15.4	5	0.65		
OVERALL	В	12.8	-	-	В	13.4	-	-		
Dean Street at Site Driveway (U)									
Dean St. NB Left	С	17.7	1	0.16	С	21.9	1	0.19		
Site Dwy. EB Right	D	29.6	3	0.49	E	35.3	3	0.49		
Kinsley Avenue at Western Site	e Drive	way (U)								
Site Dwy. NB	В	12.6	1	0.01	В	11.8	1	0.01		
Kinsley Avenue at Eastern Site	Drivev	vay (U)								
Site Dwy. NB	В	13.5	1	0.14	В	12.6	1	0.13		
(S) = Signalized										



⁽S) – Signalized (U) – Unsignalized

¹ Operates as a single junction with one controller

² Optimized Timings

^{*} Delay greater than 80 seconds.

The build alternative presented in Table 6 on the previous page along with the Kinsley Avenue directional change, takes into considerations both the base traffic growth along the servicing roadways and volumes generated by the proposed commercial redevelopment project. Under the proposed *Woonasquatucket River Greenway* project, combined with the changes in site access with restricted movements noted, the signalized junction of Dean Street/Pleasant Valley Parkway with Kinsley Avenue/Providence Place/Promenade Street will operate overall at an acceptable LOS D or better during the morning and afternoon peak periods. All critical movements will operate at LOS D or better except for the Dean Street northbound movement, Kinsley Avenue eastbound right, and Promenade Street westbound left and thru movements. During the afternoon peak period, these critical movements will continue to operate at LOS F. The unsignalized Kinsley Avenue intersection with Acorn Street continues to operate efficiently with the critical movements experiencing minor delays of fewer than 16 seconds, representing LOS C or better during the daily peak hours of traffic.

Traffic operations at the proposed full access in/right out only site driveway on Dean Street will result in minor delays for site related traffic with critical movements experiencing acceptable LOS E or better with no impacts to Dean Street through traffic during the daily morning and afternoon peak hours studied for this project. Typically, only one to two vehicles are expected to be waiting to turn left into the site from Dean Street which will be in an exclusive left turn lane that will no longer be needed for left turn access to Kinsley Avenue.

In regard to the minor site driveways on Kinsley Avenue, operations at both site driveways, which is restricted to right entering/exiting movements due to the proposed one-way eastbound movement along Kinsley Avenue, will also result in good operations with critical movements experiencing LOSB during both the daily morning and afternoon peak hours. Vehicles leaving the site destined to the north will safely exit onto Kinsley Avenue via a right turn and utilize the signalized access to Dean Street/Pleasant Valley Parkway. Typically, only one vehicle is expected to be waiting to exit at either of these site driveways on Kinsley Avenue during daily peak periods. These minor driveways have been designed to provide the geometry for the required stopping sight distances for safe access, and the appropriate length for vehicle stacking without interference from parking or turning vehicles, which should result in efficient operations, without excessive delays or driveway congestion.

6.0 CONCLUSIONS AND RECOMMENDATIONS

In summary, the study has shown that the proposed site redevelopment project access design and site circulation plan has been carefully considered to provide a level of traffic safety and efficiency on the servicing roadway system, providing multiple points of access to the adjacent street system and internal connectivity between parcels, which together will minimize travel on the local roadways. The safety of the proposed access driveways on Kinsley Avenue and on Harris Avenue were reviewed for geometry and sight distances. The proposed driveway intersections were determined to provide sufficient sight distances in accordance with AASHTO criteria for visibility and decision making of drivers attempting to enter/exit main street traffic from the proposed driveways with the proper parking restrictions as noted along the Harris Avenue frontage in the vicinity of the driveway.



An extensive review of the Dean Street access was also completed due to its proximity to high type ramp design of the right turn from Kinsley Avenue. Based upon our analysis of historical accident data obtained from the local police, field reviews of the intersection operations, and review of existing roadway geometry and physical features, it has been determined that traffic related safety enhancements could be considered at this location to improve overall operations and safety. As noted earlier in the report, the sight line visibility and higher approach speed concerns identified on the Kinsley Avenue channelized right turn approach could be mitigated with a reconfiguration of the right turn ramp to be more consistent with an urban low speed environment. This type of reconfiguration is much more appropriate today than when the intersection was originally constructed as part of the Route 6/10 interchange. The adjacent parcels are being redeveloped from manufacturing businesses adjacent to a limited access facility to more urban neighborhood businesses and residential apartments attracting pedestrian and bicycle activity along the corridors. This is exemplified with the recent upgrade of the intersection where pedestrian and bicycle accommodations including a separate bike lane along Kinsley Avenue, Providence Place and Dean Street/Pleasant Valley Parkway through the signalized intersection were added.

Summarizing our findings at this location, the study has shown that the current high speed design of the channelized right turn from Kinsley Avenue to southbound Dean Street should be modified to provide a more urban low speed design consistent with pedestrian and bicycle activity experienced in this area of the city, in addition to potentially reducing the number of yearly rear-end crashes resulting from the skewed intersection approach. Therefore, in order to provide enhanced public safety benefits for pedestrian and bicycle use of the intersection and adjacent roadways, it is recommended as part of the access design for this development project that the Kinsley Avenue channelized right turn be reconfigured with a smaller turning radius including providing a 75-foot separate channelized right turn lane from Kinsley Avenue to enhance sight lines while mitigating approach speeds. This conceptual design is shown in the Appendix for reference.

An additional concept plan has also been provided in the Appendix which depicts the access modifications necessary when the *Woonasquatucket River Greenway* project is constructed. The potential changes on Dean Street include creating a short northbound left turn lane for access into the subject property and converting the remainder of the existing northbound left turn lane at Kinsley Avenue into a raised median area. This modification will help differentiate the high type freeway interchange section of Dean Street at Route 6, to the low speed urban condition of Pleasant Valley Parkway. It allows for a refuge area for pedestrians and bicyclist crossing Dean Street and for enhanced landscape treatments that define the change in character of the environment in the vicinity of the Woonasquatucket River and Promenade Street.

The results of the operational analysis for the project determined that the estimated increase in traffic during the peak periods resulting from the proposed commercial project will have a minor effect on overall traffic operations along Dean Street, Kinsley Avenue, and Harris Avenue particularly during the daily morning and afternoon peak hours when the site services its greatest daily volumes under the current design proposal or if Kinsley Avenue becomes one-way and access modifications are made.



If the Kinsley Avenue one-way conversion is not implemented due to delays of the *Woonasquatucket River Greenway* project, it is recommended that signal timing optimization and/or phasing improvements be considered through coordination with the RIDOT as traffic volumes increase in the future at the signalized intersection of Dean Street with Pleasant Valley Parkway, Kinsley Avenue, Providence Place, and Promenade Street. As growth over the next decade may dictate additional measures, the option to provide a protected phase for the northbound left turn movement at the Kinsley Avenue intersection should be investigated, as it would address the current and future constraints experienced during the daily afternoon peak hour of traffic. As noted, this condition was designed into the recent signal reconstruction where a signal head for the left turn lane was considered in the signal design and can be implemented as volumes and operations dictate.

It is important to note that the safety mitigation recommended as part of the study would be incorporated into the current *Woonasquatucket River Greenway* extension project proposal, where the Kinsley Avenue channelized right turn will be maintained. In addition, the recommended option of providing a protected phase for the northbound left turn movement at the Dean Street intersection with Kinsley Avenue would not be necessary under the *Greenway* extension project proposal as this northbound left turn movement would be eliminated as a result of converting Kinsley Avenue from two-way traffic to a one-way eastbound movement, as previously discussed.

Therefore, based upon the data collected on the servicing roadways, the analysis completed as part of this study, along with the access design proposed, the commercial redevelopment project was determined to have adequate and safe access to a public street, and will not have an adverse impact on public safety and welfare in the study area.



APPENDIX

- A. Traffic Volume Data
- B. Traffic Crash Data
- C. Trip Generation
- D. Operational Analysis
- E. Off-Site Improvement Plan
- F. Build Alternative Plan (Woonasquatucket River Greenway)



Providence, Rhode Island

APPENDIX A - Traffic Volume Data

Automatic Traffic Recorder Count

Kinsley Avenue

Harris Avenue

Intersection Turning Movement Count

Dean Street at Kinsley Avenue/Providence Place

Pleasant Valley Parkway at Promenade Street

Kinsley Avenue at Acorn Street



Pro	posed	Comme	rcial R	edevelo	nment
	503CG	COILLIC	CIGI IX	CUCVCIO	PIIICIIC

Appendix

Providence, Rhode Island

A

Automatic Traffic Recorder Count

Kinsley Avenue

Harris Avenue



Providence, Rhode Island

Kinsley Avenue



BETA Group, Inc. 6 Blackstone Valley Place Lincoln, RI 02865

Project: Prop. Commercial Redevelopment

Town/City: Providence, RI Roadway: Kinsley Avenue Location: 150' East of Acorn Street

Start	23-Apr-18	24-Apr-18	25-Apr-18	26-Apr-18	27-Apr-18	Weekday	28-Apr-18	29-Apr-18
Time	Mon	Tue	Wed	Thu	Fri	Average	Sat	Sun
12:00 AM	*	*	44	58	58	53	91	100
01:00	*	*	36	53	62	50	72	82
02:00	*	*	29	34	31	31	76	87
03:00	*	*	32	39	32	34	53	48
04:00	*	*	73	69	72	71	46	37
05:00	*	*	155	146	162	154	66	35
06:00	*	*	288	272	262	274	132	65
07:00	*	*	472	523	475	490	165	83
08:00	*	*	610	587	544	580	203	100
09:00	*	*	355	311	297	321	228	180
10:00	*	*	280	266	303	283	277	186
11:00	*	*	296	339	322	319	263	221
12:00 PM	*	347	304	348	424	356	301	239
01:00	*	331	344	343	367	346	326	249
02:00	*	347	367	398	431	386	316	265
03:00	*	544	482	523	547	524	292	253
04:00	*	515	520	550	517	526	267	268
05:00	*	532	490	539	489	512	282	237
06:00	*	310	309	346	334	325	289	219
07:00	*	222	213	271	229	234	287	188
08:00	*	201	180	216	223	205	240	173
09:00	*	164	143	158	214	170	180	129
10:00	*	100	114	148	167	132	170	128
11:00	*	71	85	71	154	95	151	80
Total	0	3684	6221	6608	6716		4773	3652
Percentage	0.0%	56.9%	96.1%	102.1%	103.8%		73.8%	56.4%
AM Peak			08:00	08:00	08:00		10:00	11:00
Vol.			610	587	544		277	22
PM Peak		15:00	16:00	16:00	15:00		13:00	16:0
Vol.		544	520	550	547		326	268

Site Code: 599901

BETA Group, Inc. 6 Blackstone Valley Place Lincoln, RI 02865

Project: Prop. Commercial Redevelopment Town/City: Providence, RI Roadway: Kinsley Avenue Location: 150' East of Acorn Street

Start	30-Apr-18	01-May-18	02-May-18	03-May-18	04-May-18	Weekday	05-May-18	06-May-18
Time	Mon	Tue	Wed	Thu	Fri	Average	Sat	Sun
12:00 AM	58	67	*	*	*	62	*	*
01:00	42	39	*	*	*	40	*	*
02:00	24	34	*	*	*	29	*	*
03:00	35	35	*	*	*	35	*	*
04:00	64	77	*	*	*	70	*	*
05:00	147	156	*	*	*	152	*	*
06:00	249	273	*	*	*	261	*	*
07:00	491	521	*	*	*	506	*	*
08:00	600	609	*	*	*	604	*	*
09:00	322	499	*	*	*	410	*	*
10:00	238	389	*	*	*	314	*	*
11:00	251	357	*	*	*	304	*	*
12:00 PM	362	342	*	*	*	352	*	*
01:00	306	396	*	*	*	351	*	*
02:00	345	433	*	*	*	389	*	*
03:00	547	*	*	*	*	547	*	*
04:00	516	*	*	*	*	516	*	*
05:00	497	*	*	*	*	497	*	*
06:00	286	*	*	*	*	286	*	*
07:00	218	*	*	*	*	218	*	*
08:00	157	*	*	*	*	157	*	*
09:00	153	*	*	*	*	153	*	*
10:00	111	*	*	*	*	111	*	*
11:00	95	*	*	*	*	95	*	*
Total	6114	4227	0	0	0		0	0
Percentage	94.7%	65.4%	0.0%	0.0%	0.0%		0.0%	0.0%
AM Peak	08:00	08:00						
Vol.	600	609						
PM Peak	15:00	14:00						
Vol.	547	433						
Total		7911	6221					

Site Code: 599901

BETA Group, Inc. 6 Blackstone Valley Place Lincoln, RI 02865

Project: Prop. Commercial Redevelopment Town/City: Providence, RI

Roadway: Kinsley Avenue

Location:	150' Eas	t of Acorr	n Street													
Start	23-A	or-18	Tu	е	W	ed	Th	าน	F	ri	S	at	Sı	un	Week A	Average
Time	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou
12:00														•		
AM	*	*	*	*	32	12	46	12	48	10	78	13	81	19	57	13
01:00	*	*	*	*	31	5	48	5	56	6	58	14	62	20	51	10
02:00	*	*	*	*	27	2	28	6	26	5	71	5	76	11	46	6
03:00	*	*	*	*	27	5	31	8	25	7	44	9	37	11	33	8
04:00	*	*	*	*	62	11	60	9	61	11	39	7	35	2	51	8
05:00	*	*	*	*	137	18	133	13	147	15	60	6	31	4	102	11
06:00	*	*	*	*	244	44	234	38	219	43	124	8	61	4	176	27
07:00	*	*	*	*	409	63	456	67	391	84	149	16	74	9	296	48
08:00	*	*	*	*	517	93	460	127	432	112	181	22	89	11	336	73
09:00	*	*	*	*	269	86	245	66	244	53	205	23	163	17	225	49
10:00	*	*	*	*	236	44	222	44	244	59	237	40	137	49	215	47
11:00	*	*	*	*	249	47	287	52	265	57	228	35	193	28	244	44
12:00																
PM	*	*	273	74	250	54	282	66	339	85	238	63	205	34	264	63
01:00	*	*	262	69	265	79	278	65	277	90	277	49	205	44	261	66
02:00	*	*	306	41	307	60	326	72	362	69	275	41	220	45	299	55
03:00	*	*	456	88	453	29	449	74	428	119	248	44	202	51	373	68
04:00	*	*	456	59	465	55	430	120	463	54	235	32	234	34	380	59
05:00	*	*	466	66	406	84	463	76	418	71	219	63	198	39	362	66
06:00	*	*	260	50	264	45	283	63	264	70	249	40	182	37	250	51
07:00	*	*	193	29	179	34	210	61	198	31	231	56	160	28	195	40
08:00	*	*	175	26	153	27	178	38	187	36	206	34	146	27	174	31
09:00	*	*	134	30	124	19	115	43	164	50	145	35	112	17	132	32
10:00	*	*	87	13	99	15	116	32	142	25	145	25	115	13	117	20
11:00	*	*	58	13	66	19	59	12	131	23	123	28	58	22	82	20
Lane	0	0	3126	558	5271	950	5439	1169	5531	1185	4065	708	3076	576	4721	915
Day	C)	368	34	62	21	66	08	67	16	47	73	36	52	56	36
AM Peak					08:00	08:00	08:00	08:00	08:00	08:00	10:00	10:00	11:00	10:00	08:00	08:00
Vol.					517	93	460	127	432	112	237	40	193	49	336	73
PM Peak			17:00	15:00	16:00	17:00	17:00	16:00	16:00	15:00	13:00	12:00	16:00	15:00	16:00	15:00
Vol.			466	88	465	84	463	120	463	119	277	63	234	51	380	68

Site Code: 599901

Project: Prop. Commercial Redevelopment Town/City: Providence, RI Roadway: Kinsley Avenue

	150' Eas															
Start	30-Ap		T		W		Th		F		Sa			un	Week A	
Time	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou
12:00																
AM	47	11	53	14	*	*	*	*	*	*	*	*	*	*	50	12
01:00	38	4	37	2	*	*	*	*	*	*	*	*	*	*	38	3
02:00	22	2	29	5	*	*	*	*	*	*	*	*	*	*	26	4
03:00	30	5	30	5	*	*	*	*	*	*	*	*	*	*	30	5
04:00	53	11	62	15	*	*	*	*	*	*	*	*	*	*	58	13
05:00	134	13	139	17	*	*	*	*	*	*	*	*	*	*	136	15
06:00	219	30	236	37	*	*	*	*	*	*	*	*	*	*	228	34
07:00	416	75	440	81	*	*	*	*	*	*	*	*	*	*	428	78
08:00	489	111	488	121	*	*	*	*	*	*	*	*	*	*	488	116
09:00	257	65	414	85	*	*	*	*	*	*	*	*	*	*	336	75
10:00	199	39	344	45	*	*	*	*	*	*	*	*	*	*	272	42
11:00	212	39	309	48	*	*	*	*	*	*	*	*	*	*	260	44
12:00																
PM	298	64	267	75	*	*	*	*	*	*	*	*	*	*	282	70
01:00	238	68	329	67	*	*	*	*	*	*	*	*	*	*	284	68
02:00	278	67	370	63	*	*	*	*	*	*	*	*	*	*	324	65
03:00	475	72	*	*	*	*	*	*	*	*	*	*	*	*	475	72
04:00	437	79	*	*	*	*	*	*	*	*	*	*	*	*	437	79
05:00	417	80	*	*	*	*	*	*	*	*	*	*	*	*	417	80
06:00	254	32	*	*	*	*	*	*	*	*	*	*	*	*	254	32
07:00	185	33	*	*	*	*	*	*	*	*	*	*	*	*	185	33
08:00	127	30	*	*	*	*	*	*	*	*	*	*	*	*	127	30
09:00	122	31	*	*	*	*	*	*	*	*	*	*	*	*	122	31
10:00	89	22	*	*	*	*	*	*	*	*	*	*	*	*	89	22
11:00	77	18	*	*	*	*	*	*	*	*	*	*	*	*	77	18
Lane	5113	1001	3547	680	0	0	0	0	0	0	0	0	0	0	5423	1041
Day	61	14	42	27	()	()	()	0			0	64	64
AM Peak	08:00	08:00	08:00	08:00											08:00	08:00
Vol.	489	111	488	121											488	116_
PM Peak	15:00	17:00	14:00	12:00				•							15:00	17:00
Vol.	475	80	370	75											475	80
Comb. Total	61 ⁻	14	79	11	62	21	66	08	67	16	477	73	36	52	121	00
ADT	А	DT 5,894	AA	ADT 5,894												

Providence, Rhode Island

Harris Avenue



Project: Prop. Commercial Redevelopment Town/City: Providence, RI Roadway: Harris Avenue Location: 250' West of Acorn Street

Time 12:00 AM 01:00	Mon	Tue			27-Apr-18	Weekday	28-Apr-18	29-Apr-18
		Tuc	Wed	Thu	Fri	Average	Sat	Sun
01:00	•	*	*	*	*	*	123	134
	*	*	*	*	*	*	69	111
02:00	*	*	*	*	*	*	94	71
03:00	*	*	*	*	*	*	57	44
04:00	*	*	*	*	*	*	33	35
05:00	*	*	*	*	*	*	29	23
06:00	*	*	*	*	*	*	57	29
07:00	*	*	*	*	*	*	106	40
08:00	*	*	*	*	*	*	113	64
09:00	*	*	*	*	*	*	146	101
10:00	*	*	*	*	*	*	201	141
11:00	*	*	*	*	*	*	193	184
12:00 PM	*	*	*	*	297	297	241	195
01:00	*	*	*	*	278	278	235	262
02:00	*	*	*	*	314	314	226	237
03:00	*	*	*	*	365	365	240	214
04:00	*	*	*	*	475	475	244	212
05:00	*	*	*	*	489	489	302	230
06:00	*	*	*	*	344	344	264	215
07:00	*	*	*	*	261	261	277	192
08:00	*	*	*	*	239	239	256	176
09:00	*	*	*	*	231	231	202	171
10:00	*	*	*	*	179	179	182	121
11:00	*	*	*	*	157	157	170	94
Total	0	0	0	0	3629		4060	3296
Percentage	0.0%	0.0%	0.0%	0.0%	100.0%		111.9%	90.8%
AM Peak							10:00	11:00
Vol.							201	184
PM Peak					17:00		17:00	13:00
Vol.					489		302	262

Project: Prop. Commercial Redevelopment Town/City: Providence, RI Roadway: Harris Avenue Location: 250' West of Acorn Street

Start	30-Apr-18	01-May-18	02-May-18	03-May-18	04-May-18	Weekday	05-May-18	06-May-18
Time	Mon	Tue	Wed	Thu	Fri	Average	Sat	Sun
12:00 AM	77	77	79	96	103	86	113	131
01:00	33	48	39	70	53	49	92	147
02:00	20	31	25	27	36	28	83	83
03:00	31	35	30	41	23	32	39	57
04:00	39	36	52	43	58	46	41	37
05:00	43	58	68	69	65	61	33	31
06:00	114	109	105	90	87	101	57	35
07:00	308	333	308	311	247	301	79	47
08:00	350	362	340	312	281	329	119	87
09:00	226	249	198	222	210	221	156	122
10:00	209	210	190	186	183	196	186	213
11:00	195	206	207	195	189	198	214	265
12:00 PM	259	266	225	235	257	248	259	271
01:00	228	259	262	259	248	251	226	314
02:00	259	291	233	266	275	265	249	346
03:00	327	305	291	296	333	310	254	318
04:00	363	325	404	376	401	374	275	256
05:00	469	433	446	410	378	427	261	222
06:00	344	288	308	333	313	317	268	203
07:00	229	258	234	249	265	247	229	160
08:00	177	230	225	227	240	220	252	137
09:00	149	192	195	185	212	187	210	120
10:00	124	130	171	114	197	147	211	107
11:00	87	108	131	90	141	111	167	94
Total	4660	4839	4766	4702	4795		4073	3803
Percentage	98.1%	101.8%	100.3%	98.9%	100.9%		85.7%	80.0%
AM Peak	08:00	08:00	08:00	08:00	08:00		11:00	11:00
Vol.	350	362	340	312	281		214	265
PM Peak	17:00	17:00	17:00	17:00	16:00		16:00	14:00
Vol.	469	433	446	410	401		275	346
Total		4839	4766					

Project: Prop. Commercial Redevelopment Town/City: Providence, RI

Roadway: Harris Avenue

Location:	250' Wes	st of Acor	n Street													
Start		pr-18		ue	W	ed	TI	าน	F	ri	S	at	Sı	un	Week A	verage
Time	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou
12:00																
AM	*	*	*	*	*	*	*	*	*	*	5	118	11	123	8	120
01:00	*	*	*	*	*	*	*	*	*	*	3	66	15	96	9	81
02:00	*	*	*	*	*	*	*	*	*	*	6	88	15	56	10	72
03:00	*	*	*	*	*	*	*	*	*	*	1	56	5	39	3	48
04:00	*	*	*	*	*	*	*	*	*	*	0	33	5	30	2	32
05:00	*	*	*	*	*	*	*	*	*	*	2	27	1	22	2	24
06:00	*	*	*	*	*	*	*	*	*	*	3	54	4	25	4	40
07:00	*	*	*	*	*	*	*	*	*	*	8	98	7	33	8	66
08:00	*	*	*	*	*	*	*	*	*	*	2	111	8	56	5	84
09:00	*	*	*	*	*	*	*	*	*	*	1	145	12	89	6	117
10:00	*	*	*	*	*	*	*	*	*	*	8	193	22	119	15	156
11:00	*	*	*	*	*	*	*	*	*	*	7	186	34	150	20	168
12:00																
PM	*	*	*	*	*	*	*	*	56	241	15	226	48	147	40	205
01:00	*	*	*	*	*	*	*	*	69	209	47	188	62	200	59	199
02:00	*	*	*	*	*	*	*	*	57	257	51	175	59	178	56	203
03:00	*	*	*	*	*	*	*	*	90	275	39	201	48	166	59	214
04:00	*	*	*	*	*	*	*	*	107	368	39	205	44	168	63	247
05:00	*	*	*	*	*	*	*	*	110	379	64	238	47	183	74	267
06:00	*	*	*	*	*	*	*	*	38	306	50	214	40	175	43	232
07:00	*	*	*	*	*	*	*	*	34	227	53	224	26	166	38	206
08:00	*	*	*	*	*	*	*	*	22	217	56	200	18	158	32	192
09:00	*	*	*	*	*	*	*	*	17	214	27	175	21	150	22	180
10:00	*	*	*	*	*	*	*	*	12	167	24	158	16	105	17	143
11:00	*	*	*	*	*	*	*	*	11	146	25	145	7	87	14	126
Lane	0	0	0	0	0	0	0	0	623	3006	536	3524	575	2721	609	3422
Day	()	(0	(0	()	36	29	40	60	32	96	403	31
AM Peak											07:00	10:00	11:00	11:00	11:00	11:00
Vol.											8	193	34	150	20	168
PM Peak									17:00	17:00	17:00	17:00	13:00	13:00	17:00	17:00
Vol.									110	379	64	238	62	200	74	267

Project: Prop. Commercial Redevelopment

Town/City: Providence, RI Roadway: Harris Avenue Location: 250' West of Acorn Street

Location:	250' Wes	t of Acor														
Start	30-Ap	or-18	Ti	ue	W	ed	TI	hu	F	ri	S	at	S	un	Week A	Average
Time	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou	Eastbou	Westbou
12:00																
AM	11	66	11	66	9	70	22	74	33	70	18	95	28	103	19	78
01:00	1	32	6	42	5	34	10	60	8	45	24	68	27	120	12	57
02:00	1	19	4	27	2	23	2	25	1	35	28	55	21	62	8	35
03:00	3	28	4	31	8	22	6	35	2	21	5	34	8	49	5	31
04:00	7	32	4	32	6	46	4	39	9	49	8	33	6	31	6	37
05:00	14	29	10	48	11	57	16	53	14	51	4	29	8	23	11	41
06:00	23	91	26	83	20	85	13	77	6	81	20	37	16	19	18	68
07:00	62	246	65	268	83	225	45	266	38	209	21	58	9	38	46	187
08:00	57	293	71	291	145	195	42	270	46	235	29	90	23	64	59	205
09:00	40	186	40	209	46	152	44	178	42	168	31	125	29	93	39	159
10:00	34	175	40	170	59	131	39	147	20	163	35	151	54	159	40	157
11:00	31	164	34	172	42	165	31	164	35	154	59	155	50	215	40	170
12:00																
PM	39	220	50	216	26	199	36	199	36	221	52	207	45	226	41	213
01:00	36	192	38	221	15	247	38	221	32	216	39	187	54	260	36	221
02:00	42	217	51	240	17	216	34	232	35	240	66	183	62	284	44	230
03:00	62	265	70	235	19	272	29	267	17	316	64	190	52	266	45	259
04:00	65	298	60	265	14	390	57	319	22	379	65	210	76	180	51	292
05:00	62	407	85	348	37	409	46	364	54	324	56	205	44	178	55	319
06:00	60	284	50	238	35	273	34	299	39	274	61	207	36	167	45	249
07:00	32	197	26	232	23	211	29	220	38	227	38	191	10	150	28	204
08:00	20	157	30	200	14	211	17	210	25	215	49	203	26	111	26	187
09:00	18	131	35	157	7	188	28	157	19	193	36	174	28	92	24	156
10:00	14	110	17	113	15	156	21	93	8	189	34	177	24	83	19	132
11:00	10	77	10	98	12	119	20	70	16	125	39	128	21	73	18	99
Lane	744	3916	837	4002	670	4096	663	4039	595	4200	881	3192	757	3046	735	3786
Day	460	60	48	39	47	'66	47	02	47	95	40	73	38	803	45	
AM Peak	07:00	08:00	08:00	08:00	08:00	07:00	07:00	08:00	08:00	08:00	11:00	11:00	10:00	11:00	08:00	08:00
Vol.	62	293	71	291	145	225	45	270	46	235	59	155	54	215	59	205
PM Peak	16:00	17:00	17:00	17:00	17:00	17:00	16:00	17:00	17:00	16:00	14:00	16:00	16:00	14:00	17:00	17:00
Vol.	65	407	85	348	37	409	57	364	54	379	66	210	76	284	55	319
Comb. Total	466	60	48	39	47	66	47	02	84	24	81	33	70	99	85	52
ADT	А	DT 4,488	AA	ADT 4,488												

Pro	oosed	Commercial	Redevelo	pment

Appendix

Providence, Rhode Island

A

Intersection Turning Movement Count

Dean Street at Kinsley Avenue/Providence Place
Pleasant Valley Parkway at Promenade Street
Kinsley Avenue at Acorn Street



Providence, Rhode Island

Dean Street at Kinsley Avenue/Providence Place



Client: Amy Black
Project #: 194_010_BETA
BTD #: Location 2
Location: Providence, RI
Street 1: Dean Street

Street 2: Kinsley Avenue/Providence Place

Count Date: 4/24/2018
Day of Week: Tuesday
Weather: Partly Sunny, 65°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TOTAL (CARS & TRUCKS)

		Dane	Ctunnat			Dana	C4===4	•		Í/:malau	A.,			Dunidan	as Disas	
			Street				Street			rinsiey	Avenue				ice Place	
		North	nbound			South	bound			Eastl	oound			West	bound	
Start Time	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	14	194	67	129	0	3	237	2	0	4	7	75	0	0	0	0
7:15 AM	18	199	81	132	0	4	243	2	0	5	6	78	0	0	0	0
7:30 AM	21	195	92	134	0	8	254	3	0	6	11	86	0	0	0	0
7:45 AM	24	197	107	137	0	12	261	3	0	7	16	91	0	0	0	0
8:00 AM	26	196	118	139	0	16	274	2	0	8	21	98	0	0	0	0
8:15 AM	27	194	115	140	0	19	277	3	0	9	25	102	0	0	0	0
8:30 AM	24	190	112	135	0	17	275	3	0	9	24	99	0	0	0	0
8:45 AM	22	188	109	132	0	16	271	2	0	8	22	97	0	0	0	0

			Street				Street bound			•	Avenue				nce Place bound	
Start Time	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	11	262	75	80	0	24	347	1	0	3	13	58	0	0	0	0
4:15 PM	9	273	80	89	0	25	361	2	0	4	14	60	0	0	0	0
4:30 PM	10	277	81	94	0	26	367	1	0	5	18	67	0	0	0	0
4:45 PM	10	286	85	103	0	24	371	2	0	6	21	71	0	0	0	0
5:00 PM	9	287	86	107	0	23	359	2	0	8	25	78	0	0	0	0
5:15 PM	10	288	85	105	0	22	336	1	0	9	28	81	0	0	0	0
5:30 PM	8	282	83	102	0	26	327	2	0	8	27	79	0	0	0	0
5:45 PM	9	280	84	98	0	23	324	1	0	7	25	77	0	0	0	0

AM PEAK HOUR 8:00 AM			Street				Street bound			,	Avenue				nce Place bound	
to	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
9:00 AM	99	768	454	546	0	68	1097	10	0	34	92	396	0	0	0	0
PHF		0	.97			0.	98			0.	96			0.	00	
HV%	1.0%	0.3%	0.4%	0.2%	0.0%	1.5%	0.9%	0.0%	0.0%	0.0%	0.0%	0.5%	0.0%	0.0%	0.0%	0.0%

PM PF	EAK HOUR		Dean	Street			Dean	Street			Kinsley	Avenue			Provider	nce Place	
4:	:30 PM		North	nbound			South	bound			Eastl	oound			West	bound	
	to	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:	:30 PM	39	1138	337	409	0	95	1433	6	0	28	92	297	0	0	0	0
	PHF		0	.98			0.	97			0.	88			0.	.00	
1	HV %	0.0%	0.3%	0.0%	0.0%	0.0%	0.0%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

Note:

1. Dean Street Northbound Right (I-95) movements are vehicles traveling Dean Street Northbound, from I-95 Northbound, take right onto Providence Place.

Client: Amy Black
Project #: 194_010_BETA
BTD #: Location 2
Location: Providence, RI
Street 1: Dean Street

Street 2: Kinsley Avenue/Providence Place

Count Date: 4/24/2018
Day of Week: Tuesday
Weather: Partly Sunny, 65°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TRUCKS

		Dean	Street			Dean	Street			Kinsley	Avenue			Provider	nce Place	
		North	nbound			South	bound			Eastl	bound			West	bound	
Start Time	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	1	0	0	2	0	0	0	0	0	0	0	0	0
7:15 AM	1	1	0	1	0	0	5	0	0	0	0	1	0	0	0	0
7:30 AM	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
8:00 AM	0	1	1	0	0	1	2	0	0	0	0	1	0	0	0	0
8:15 AM	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0
8:45 AM	1	1	1	1	0	0	3	0	0	0	0	1	0	0	0	0

			Street				Street bound				Avenue				ice Place bound	
Start Time	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	1	0	0	0	0	2	0	0	0	0	0	0	0	0	0
4:45 PM	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0
5:00 PM	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

AM PEAK HOUR		Dean	Street			Dean	Street			Kinsley	Avenue			Providen	ce Place	
7:15 AM		North	bound			South	bound			Easth	oound			Westl	oound	
to	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:15 AM	1	2	1	1	0	1	11	0	0	0	0	2	0	0	0	0
PHF		0	.42		•	0.0	60			0.	50	•		0.	00	

PM PEAK HOUR		Dean	Street			Dean	Street			Kinsley	Avenue			Providen	ce Place	
4:15 PM		North	bound			South	bound			Eastb	ound			Westh	oound	
to	Left	Thru	Right	Right (I-95)	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:15 PM	0	4	0	0	0	0	4	0	0	0	0	0	0	0	0	0
PHF		1	.00			0.	50			0.0	00			0.0	00	

Client: Amy Black
Project #: 194_010_BETA
BTD #: Location 2
Location: Providence, RI
Street 1: Dean Street

Street 2: Kinsley Avenue/Providence Place

Count Date: 4/24/2018
Day of Week: Tuesday
Weather: Partly Sunny, 65°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

PEDESTRIANS & BICYCLES

			Dean Stree Northbound				Dean Stree				insley Aven Eastbound				vidence Pla Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	
7:15 AM	0	0	0	1	0	0	0	0	0	1	0	4	0	0	0	0	
7:30 AM	0	0	0	0	0	1	0	0	0	0	0	2	0	0	0	1	
7:45 AM	0	0	0	1	0	1	0	0	0	0	0	1	0	0	0	0	
8:00 AM	0	0	0	0	0	1	0	0	0	0	0	2	0	0	0	0	
8:15 AM	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	2	
8:30 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM	0	0	0	0	0	1	0	0	0	2	0	2	0	0	0	0	

			Dean Stree Northbound				Dean Stree				nsley Aven Eastbound				vidence Pla Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
4:15 PM	0	0	0	2	0	0	0	0	0	0	0	2	0	0	0	1	
4:30 PM	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	
4:45 PM	0	1	0	2	0	0	0	0	0	0	0	1	0	0	0	0	
5:00 PM	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	1	0	0	0	0	0	0	0	2	0	0	0	0	
5:45 PM	0	0	0	3	0	0	0	0	0	2	0	1	0	0	0	0	

AM PEAK HOUR ¹]		Dean Stree	t				Dean Stree	t		Ki	insley Aven	ue		Pro	ovidence Pla	ace	
8:00 AM			Northbound	l				Southbound	i			Eastbound				Westbound	i	
to	Left	Northbound					Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
9:00 AM	0	0	0	2		0	2	0	0	0	2	0	5	0	0	0	2	

PM PEAK HOUR ¹ 4:30 PM								Dean Stree	t d		Ki	insley Aven Eastbound				ovidence Pla Westbound		
to	Dean Street Northbound Left Thru Right PED 0 1 0 4					Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
5:30 PM	0	1	0	4		0	1	0	0	0	0	0	2	0	0	0	0	

Peak hours corresponds to vehicular peak hours.

Providence, Rhode Island

Pleasant Valley Parkway at Promenade Street



Client: Amy Black
Project #: 194_010_BETA
BTD #: Location 1
Location: Providence, RI

Street 1: Pleasant Valley Parkway/Dean Street

Street 2: Promenade Street
Count Date: 4/24/2018
Day of Week: Tuesday
Weather: Partly Sunny, 65°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TOTAL (CARS & TRUCKS)

										,						
		Dean	Street		F	Pleasant Va	lley Parkwa	y		Promena	de Street			Promena	de Street	
		North	bound			South	bound			Eastl	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	11	187	0	0	0	213	6	0	0	0	0	0	29	30	64
7:15 AM	AM 0 11 187 AM 0 14 190 AM 0 12 189 AM 0 13 191		0	0	0	216	7	0	0	0	0	0	33	34	66	
7:30 AM	0	12	189	0	0	0	229	8	0	0	0	0	0	36	37	65
7:45 AM	0	13	191	0	0	0	237	8	0	0	0	0	0	39	41	67
8:00 AM	0	12	192	0	0	0	250	9	0	0	0	0	0	42	45	66
8:15 AM	0	13	190	0	0	0	258	8	0	0	0	0	0	41	44	65
8:30 AM	0	11	188	0	0	0	255	7	0	0	0	0	0	40	43	63
8:45 AM	0	10	186	0	0	0	251	7	0	0	0	0	0	38	42	61

		Dean	Street		F	Pleasant Va	lley Parkwa	y		Promena	ide Street			Promena	de Street	
		North	bound			South	bound			Easth	oound			Westl	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	7	258	0	0	0	247	9	0	0	0	0	0	125	24	41
4:15 PM	0	8	269	0	0	0	254	10	0	0	0	0	0	134	29	48
4:30 PM	0	8	274	0	0	0	253	8	0	0	0	0	0	141	33	54
4:45 PM	0	9	283	0	0	0	247	9	0	0	0	0	0	150	37	63
5:00 PM	0	8	287	0	0	0	227	10	0	0	0	0	0	157	40	69
5:15 PM	0	9	288	0	0	0	203	8	0	0	0	0	0	156	39	68
5:30 PM	0	7	283	0	0	0	201	9	0	0	0	0	0	154	37	65
5:45 PM	0	8	279	0	0	0	197	8	0	0	0	0	0	151	35	63

AM PEAK HOU	IR .	Dean	Street		F	Pleasant Va	lley Parkwa	у		Promena	de Street			Promena	de Street	
7:45 AM		North	bound			South	bound			Easth	oound			Westl	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:45 AM	0	0 49 761 0				0	1000	32	0	0	0	0	0	162	173	261
PHF		0.	.99			0.	97			0.	00			0.	97	
HV %	0.0%	0.99				0.0%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	3.1%	0.6%	1.5%

PM PEAK HOUL	1	Dean	Street		F	Pleasant Va	lley Parkwa	у		Promena	de Street			Promena	de Street	
4:30 PM		North	bound			South	bound	-		Eastb	ound			Westl	bound	
to	U-Turn	U-Turn Left Thru Right				Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:30 PM	0	34	1132	0	0	0	930	35	0	0	0	0	0	604	149	254
PHF		0.	.98			0.	92			0.	00			0.	95	
HV~%	0.0%	0.0%	0.3%	0.0%	0.0%	0.0%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.3%	0.7%	1.2%

Client: Amy Black
Project #: 194_010_BETA
BTD #: Location 1
Location: Providence, RI

Street 1: Pleasant Valley Parkway/Dean Street

Street 2: Promenade Street
Count Date: 4/24/2018
Day of Week: Tuesday
Weather: Partly Sunny, 65°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TRUCKS

								,,,,	0,10							
		Dean	Street		F	Pleasant Va	illey Parkwa	ıy		Promena	de Street			Promena	de Street	
		North	bound			South	bound			Easth	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	0	2	0	0	0	0	0	0	0	1	1
7:15 AM	0	0	1	0	0	0	1	0	0	0	0	0	0	4	0	1
7:30 AM	0	0	0	0	0	0	2	0	0	0	0	0	0	1	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1
8:00 AM	0	0	1	0	0	0	1	0	0	0	0	0	0	2	0	1
8:15 AM	0	0	0	0	0	0	2	0	0	0	0	0	0	1	0	0
8:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	2
8:45 AM	0	0	1	0	0	0	2	0	0	0	0	0	0	1	0	0

		Dean	Street		F	Pleasant Va	illey Parkwa	y		Promena	de Street			Promena	ade Street	
		North	bound			South	bound			Eastl	bound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
4:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	1	0	0	0	1	0	0	0	0	0	0	1	0	1
4:45 PM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	1
5:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:30 PM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Γ	AM PEAK HOUR		Dean	Street		P	leasant Va	lley Parkwa	y		Promena	de Street			Promena	de Street	
	7:15 AM		North	oound			South	bound			Easth	oound			Westl	oound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	8:15 AM	0	0	2	0	0	0	4	0	0	0	0	0	0	8	1	3
_	PHF		0.	50			0.	50			0.	00			0.	60	

PM I	PEAK HOUR		Dean	Street		P	leasant Va	lley Parkwa	У		Promena	de Street			Promena	de Street	
	4:15 PM		North	bound			South	bound			Easth	oound			West	oound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	5:15 PM	0	0	4	0	0	0	2	0	0	0	0	0	0	2	1	2
	PHF		1.	00			0.	50			0.	00			0.	63	

Client: Amy Black
Project #: 194_010_BETA
BTD #: Location 1
Location: Providence, RI

Street 1: Pleasant Valley Parkway/Dean Street

Street 2: Promenade Street
Count Date: 4/24/2018
Day of Week: Tuesday
Weather: Partly Sunny, 65°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

PEDESTRIANS & BICYCLES

			Dean Stree	t		Pleasa	ant Valley P	arkway		Pro	menade St	reet		Pro	menade St	reet	
			Northbound	l			Southbound	b			Eastbound				Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	1	0	0	0	3	0	1	0	0	
7:30 AM	0	0	0	0	0	1	0	0	0	0	0	2	0	0	0	1	
7:45 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
8:00 AM	0	0	0	0	0	1	0	4	0	0	0	3	0	0	0	0	
8:15 AM	0	0	0	0	0	0	0	3	0	0	0	1	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:45 AM	0	0	0	0	0	1	0	0	0	0	0	2	0	0	0	0	

			Dean Stree Northbound			Pleasa	ant Valley P	arkway d			menade St				menade Sti Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
4:30 PM	0	0	0	0	0	1	0	3	0	0	0	0	0	0	0	1	
4:45 PM	0	1	0	0	0	0	0	2	0	0	0	1	0	0	0	0	
5:00 PM	0	0	0	0	0	0	0	5	0	0	0	1	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	1	0	0	0	2	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	2	0	0	0	1	0	1	0	0	

AM PEAK HOUR ¹]		Dean Stree			Pleasa	ant Valley Pa	arkway		Pro	menade St	reet			menade St		
7:45 AM			Northbound	l			Southbound	i			Eastbound				Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
8:45 AM	0	0	0	0	0	2	0	7	0	0	0	4	0	0	0	1	

PM PEAK HOUR ¹			Dean Stree			Pleasa	ant Valley P	arkway		Pro	menade St	reet		Pro	menade Sti	reet	
4:30 PM			Northbound				Southbound	i			Eastbound				Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
5:30 PM	0	1	0	0	0	1	0	12	0	0	0	2	0	0	0	1	

Peak hours corresponds to vehicular peak hours.

Providence, Rhode Island

Kinsley Avenue at Acorn Street





6 Blackstone Valley Place Lincoln, RI 02865

Project: Prop. Commercial Redevelopment

Town/City: Providence, RI

Location: Kinsley Ave. at Acorn St.

Weather: Sunny/60's

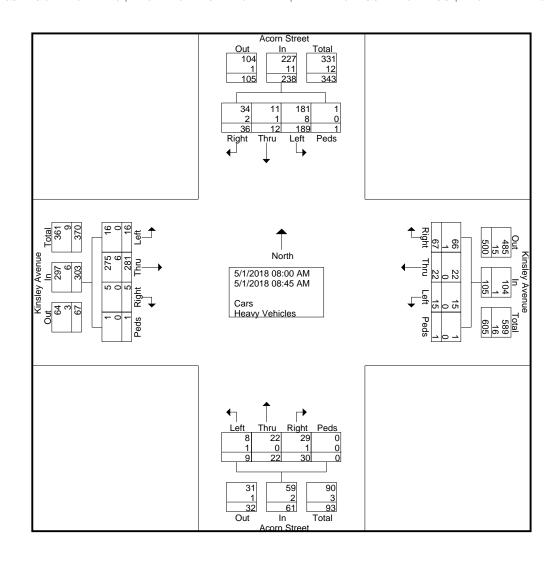
File Name: Acorn at Kinsley AM

Site Code : 59991 Start Date : 5/1/2018

Page No : 1

Groups Printed- Cars - Heavy Vehicles

								Or Oup	<i>3</i> 1 111110	u cuis	Houry	, cilici	C D								
		A	corn Stı	eet			Kin	sley Av	enue			A	corn St	reet			Kin	sley Av	enue		
		Se	outhbou	ınd			V	estbou	nd			N	orthbou	ınd			E	Eastbour	nd		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
08:00 AM	72	3	16	0	91	5	9	24	1	39	3	5	8	0	16	7	75	0	0	82	228
08:15 AM	61	4	7	0	72	4	7	13	0	24	1	6	8	0	15	4	76	1	0	81	192
08:30 AM	30	3	7	1	41	4	2	17	0	23	1	5	10	0	16	4	63	2	1	70	150
08:45 AM	26	2	6	0	34	2	4	13	0	19	4	6	4	0	14	1	67	2	0	70	137
Total	189	12	36	1	238	15	22	67	1	105	9	22	30	0	61	16	281	5	1	303	707
Grand Total	189	12	36	1	238	15	22	67	1	105	9	22	30	0	61	16	281	5	1	303	707
Apprch %	79.4	5	15.1	0.4		14.3	21	63.8	1		14.8	36.1	49.2	0		5.3	92.7	1.7	0.3		
Total %	26.7	1.7	5.1	0.1	33.7	2.1	3.1	9.5	0.1	14.9	1.3	3.1	4.2	0	8.6	2.3	39.7	0.7	0.1	42.9	
Cars	181	11	34	1	227	15	22	66	1	104	8	22	29	0	59	16	275	5	1	297	687
% Cars	95.8	91.7	94.4	100	95.4	100	100	98.5	100	99	88.9	100	96.7	0	96.7	100	97.9	100	100	98	97.2
Heavy Vehicles	8	1	2	0	11	0	0	1	0	1	1	0	1	0	2	0	6	0	0	6	20
% Heavy Vehicles	4.2	8.3	5.6	0	4.6	0	0	1.5	0	1	11.1	0	3.3	0	3.3	0	2.1	0	0	2	2.8





Project: Prop. Commercial Redevelopment

Town/City: Providence, RI

Location: Kinsley Ave. at Acorn St.

Weather: Sunny/60's

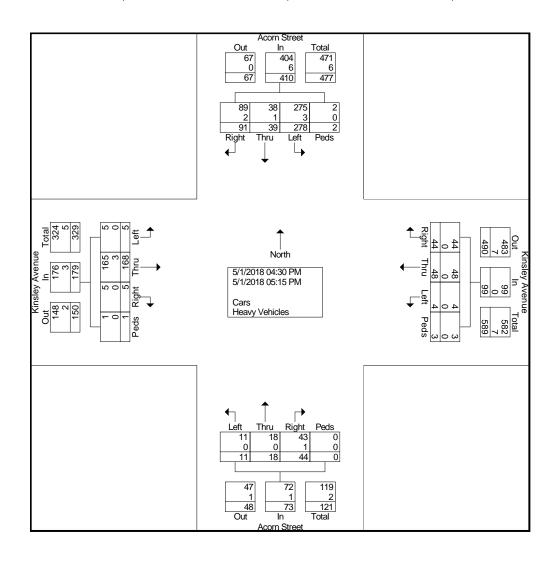
File Name: Acorn at Kinsley PM

Site Code : 59992 Start Date : 5/1/2018

Page No : 1

Groups Printed- Cars - Heavy Vehicles

			corn St					sley Av /estbou					corn St					sley Av			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
04:30 PM	84	9	25	0	118	0	7	8	0	15	4	2	14	0	20	4	51	1	0	56	209
04:45 PM	59	14	14	2	89	2	16	11	0	29	3	6	11	0	20	1	38	3	1	43	181
Total	143	23	39	2	207	2	23	19	0	44	7	8	25	0	40	5	89	4	1	99	390
05:00 PM	73	5	28	0	106	1	11	10	0	22	3	6	15	0	24	0	49	1	0	50	202
05:15 PM	62	11	24	0	97	1	14	15	3	33	1	4	4	0	9	0	30	0	0	30	169
Grand Total	278	39	91	2	410	4	48	44	3	99	11	18	44	0	73	5	168	5	1	179	761
Apprch %	67.8	9.5	22.2	0.5		4	48.5	44.4	3		15.1	24.7	60.3	0		2.8	93.9	2.8	0.6		
Total %	36.5	5.1	12	0.3	53.9	0.5	6.3	5.8	0.4	13	1.4	2.4	5.8	0	9.6	0.7	22.1	0.7	0.1	23.5	
Cars	275	38	89	2	404	4	48	44	3	99	11	18	43	0	72	5	165	5	1	176	751
% Cars	98.9	97.4	97.8	100	98.5	100	100	100	100	100	100	100	97.7	0	98.6	100	98.2	100	100	98.3	98.7
Heavy Vehicles	3	1	2	0	6	0	0	0	0	0	0	0	1	0	1	0	3	0	0	3	10
% Heavy Vehicles	1.1	2.6	2.2	0	1.5	0	0	0	0	0	0	0	2.3	0	1.4	0	1.8	0	0	1.7	1.3



Providence, Rhode Island

ATTACHMENT B - Traffic Crash Data

Crash Summary

January 2017 through December 2018

(Dean Street at Kinsley Avenue)

Crash Diagrams

Dean Street at Kinsley Avenue

Crash Reports

Dean Street at Kinsley Avenue (Channelized EB Right Turn)



Propo	sed Con	nmercial	Redevel	opment
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Appendix

Providence, Rhode Island

В

Crash Summary

Dean Street at Kinsley Avenue

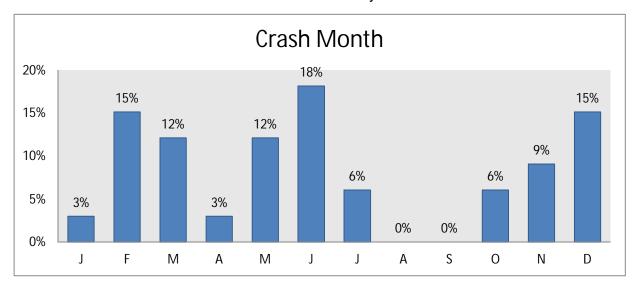
Crash Data Summary Table Dean Street at Kinsley Avenue

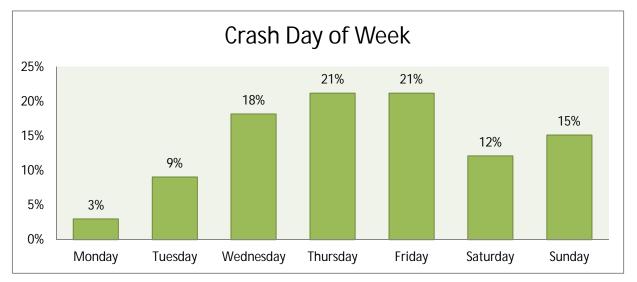
1/1/2017 - 12/31/2018

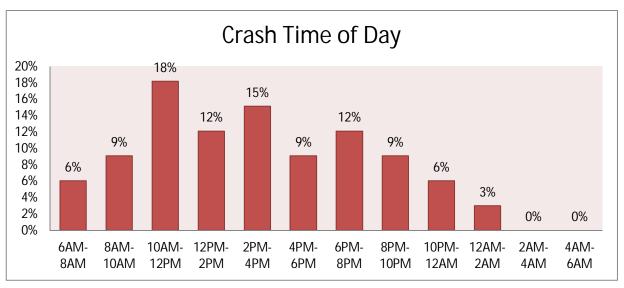
						Dead		
Ref#	Crash Date	Day	Crash Time	Collision Type	Light Condition	Road Condition	Severity	Notes
1	7/19/2018	Thursday	6:04 PM	Angle	Daylight	Dry	Injury	unclear report
2	6/8/2017	Thursday	10:12 PM	Sideswipe, Same Direction	Dark - Lighted	Dry	Property	attempted illegal pass while turning right
3	11/3/2017	Friday	10:25 AM	Sideswipe, Same Direction	Daylight	Dry	Property	attempted megal pase trime tarring right
4	3/19/2018	Monday	8:47 AM	Rear End	Daylight	Dry	Injury	stopped at yield sign
5	12/29/2017	Friday	1:57 PM	Rear End	Daylight	Dry	Property	stopped at yield sign, looking at traffic to left
6	2/27/2018	Tuesday	7:33 AM	Rear End	Daylight	Dry	Injury	stopped at yield sign, looking at traffic to left
7	3/16/2018	Friday	8:48 AM	Rear End	Daylight	Dry	Property	stopped at yield sign, looking at traffic to left
8	6/24/2018	Sunday	11:27 AM	Rear End	Daylight	Dry	Property	stopped at yield sign
9	2/28/2018	Wednesday	11:58 AM	Rear End	Daylight	Dry	Property	Stopped at light south
10	3/6/2018	Tuesday	10:18 AM	Rear End	Daylight	Dry	Injury	Stopped in traffic south
11	11/8/2018	Thursday	8:53 AM	Rear End	Daylight	Dry	Injury	Slowed in traffic south
12	2/2/2018	Friday	11:23 AM	Rear End	Daylight	Wet	Injury	Stopped at light south
13	6/17/2018	Sunday	4:38 PM	Rear End	Daylight	Dry	Injury	
14	12/22/2017	Friday	3:04 PM	Rear End	Daylight	Dry	Property	Stopped in traffic south
15	6/17/2017	Saturday	12:54 PM	Sideswipe, Same Direction	Daylight	Dry	Property	construction
16	1/31/2017	Tuesday	2:58 PM	Rear End	Daylight	Snow	Property	Slid on ice/ snow
17	11/12/2017	Sunday	11:12 AM	Rear End	Daylight	Dry	Property	Stopped at light
18	12/21/2017	Thursday	8:52 PM	Rear End	Dark - Lighted	Dry	Property	Stopped at light north
19	10/24/2018	Wednesday	3:17 PM	Rear End	Daylight	Dry	Property	Stopped at light south
20	2/17/2018	Saturday	5:26 PM	Angle	Dark - Lighted	Dry	Property	turning left onto kinsley west
21	3/10/2018	Saturday	12:07 PM	Angle	Daylight	Dry	Injury	turning left onto kinsley east
22	5/9/2018	Wednesday	9:31 PM	Angle	Dark - Lighted	Dry	Injury	speeding, turning left onto kinsley east
23	6/10/2018	Sunday	3:41 PM	Angle	Daylight	Dry	Property	Ran red light
24	5/17/2018	Thursday	5:00 PM	Rear End	Daylight	Dry	Property	
25	5/11/2018	Friday	9:50 PM	Sideswipe, Same Direction	Dark - Lighted	Dry	Property	Both turning left onto dean st
26	5/20/2018	Sunday	2:55 PM	Angle	Daylight	Dry	Property	turning left onto kinsley west
27	7/19/2017	Wednesday	7:35 PM	Sideswipe, Same Direction	Dusk	Dry	Property	Both turning right onto kinsley east
28	10/12/2017	Thursday	12:03 PM	Rear End	Daylight	Dry	Injury	vehicle waiting to turn left
29	12/28/2017	Thursday	6:57 PM	Head-On	Dark - Lighted	Dry	Injury	turning left onto kinsley east
30	6/30/2018	Saturday	1:24 AM	Rear End	Dark - Lighted	Dry	Property	Stopped at light north
31	12/28/2018	Friday	6:03 PM	Angle	Dark - Lighted	Wet	Property	Changing lanes north
32	2/14/2018	Wednesday	10:35 PM	Angle	Dark - Lighted	Dry	Property	Turning left onto Pleasant valley south
33	4/25/2018	Wednesday	7:51 AM	Sideswipe, Same Direction	Daylight	Wet	Property	Changing lanes north

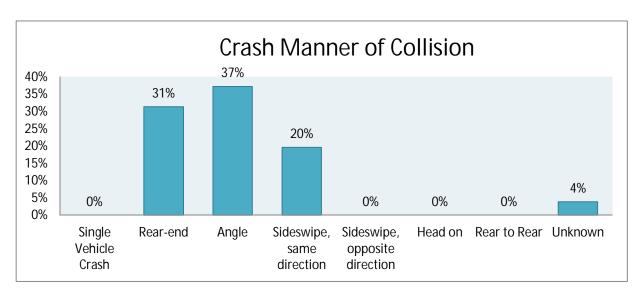
Crash Data Summary Tables and Charts

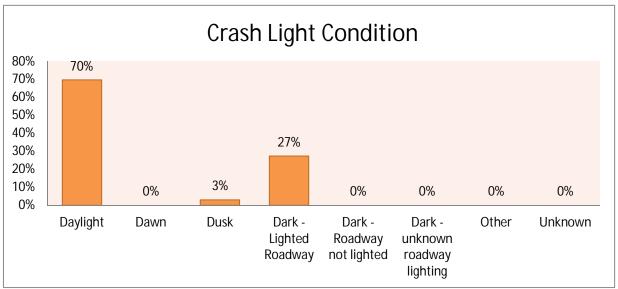
Dean Street at Kinsley Ave

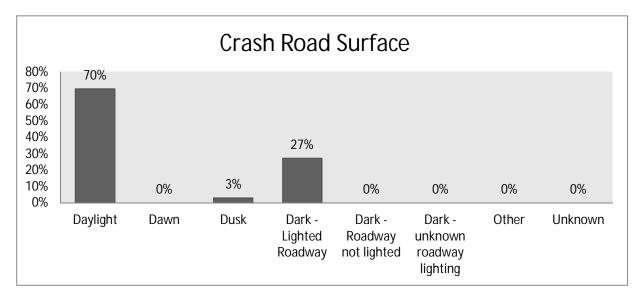












Kinsley @ Dean St (Signal)

	2017	2018	Total	Percent
Collision Type				
Rear End	5	8	13	52%
Angle	0	7	7	28%
Head-On	1	0	1	4%
Pedestrian	0	0	0	0%
Sideswipe, Same Direction	2	2	4	16%
Sideswipe, Opposite Direction	0	0	0	0%
Collision with Object	0	0	0	0%
Accident Severity				
Property	6	11	17	68%
Injury	2	6	8	32%
Fatal	0	0	0	0%
Light Condition				
Daylight	5	11	16	64%
Dawn	0	0	0	0%
Dusk	1	0	1	4%
Dark - Lighted	2	6	8	32%
Dark - Not Lighted	0	0	0	0%
Other	0	0	0	0%
Unknown	0	0	0	0%
Road Condition				
Dry	7	14	21	84%
Wet	0	3	3	12%
Snow	1	0	1	4%
Slush	0	0	0	0%
Ice/Frost	0	0	0	0%
Other	0	0	0	0%
Unknown	0	0	0	0%
Hour of Day				
6:00 AM - 9:00 AM	0	2	2	8%
9:00 AM - 3:00 PM	4	5	9	36%
3:00 PM - 6:00 PM	1	5	6	24%
6:00 PM - 6:00 AM	3	5	8	32%
Total Accidents:	8	17	25	



Kinsley Channelized Right @ Dean St (Yield)

	2017	2018	Total	Percent
Collision Type				
Rear End	1	4	5	63%
Angle	0	1	1	13%
Head-On	0	0	0	0%
Pedestrian	0	0	0	0%
Sideswipe, Same Direction	2	0	2	25%
	0	0	0	0%
Sideswipe, Opposite Direction		-	-	
Collision with Object	0	0	0	0%
Accident Severity				
Property	3	2	5	63%
Injury	0	3	3	38%
Fatal	0	0	0	0%
Light Condition				
Daylight	2	5	7	88%
Dawn	0	0	0	0%
Dusk	0	0	0	0%
Dark - Lighted	1	0	1	13%
Dark - Not Lighted	0	0	0	0%
Other	0	0	0	0%
Unknown	0	0	0	0%
Road Condition				
Dry	3	5	8	100%
Wet	0	0	0	0%
Snow	0	0	0	0%
Slush	0	0	0	0%
Ice/Frost	0	0	0	0%
Other	0	0	0	0%
Unknown	0	0	0	0%
Hour of Day				
6:00 AM - 9:00 AM	0	3	3	38%
9:00 AM - 3:00 PM	2	1	3	38%
3:00 PM - 6:00 PM	0	0	0	0%
6:00 PM - 6:00 AM	1	1	2	
U.UU PIVI - U.UU AIVI	1	1		25%
Total Accidents:	3	5	8	



Proposed	d Commercial	Redevelo	pment
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Appendix

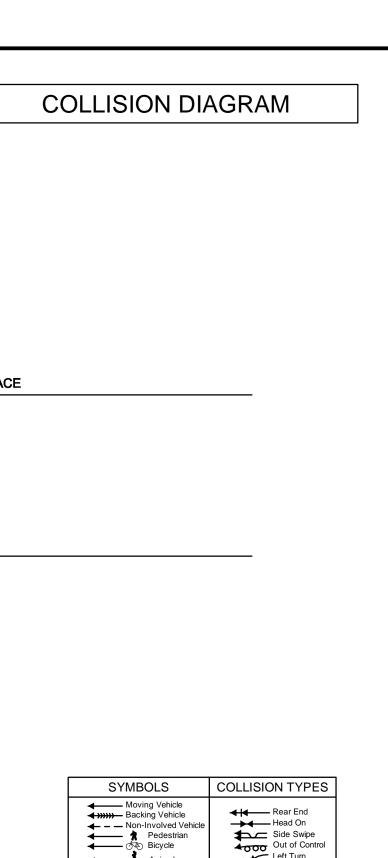
Providence, Rhode Island

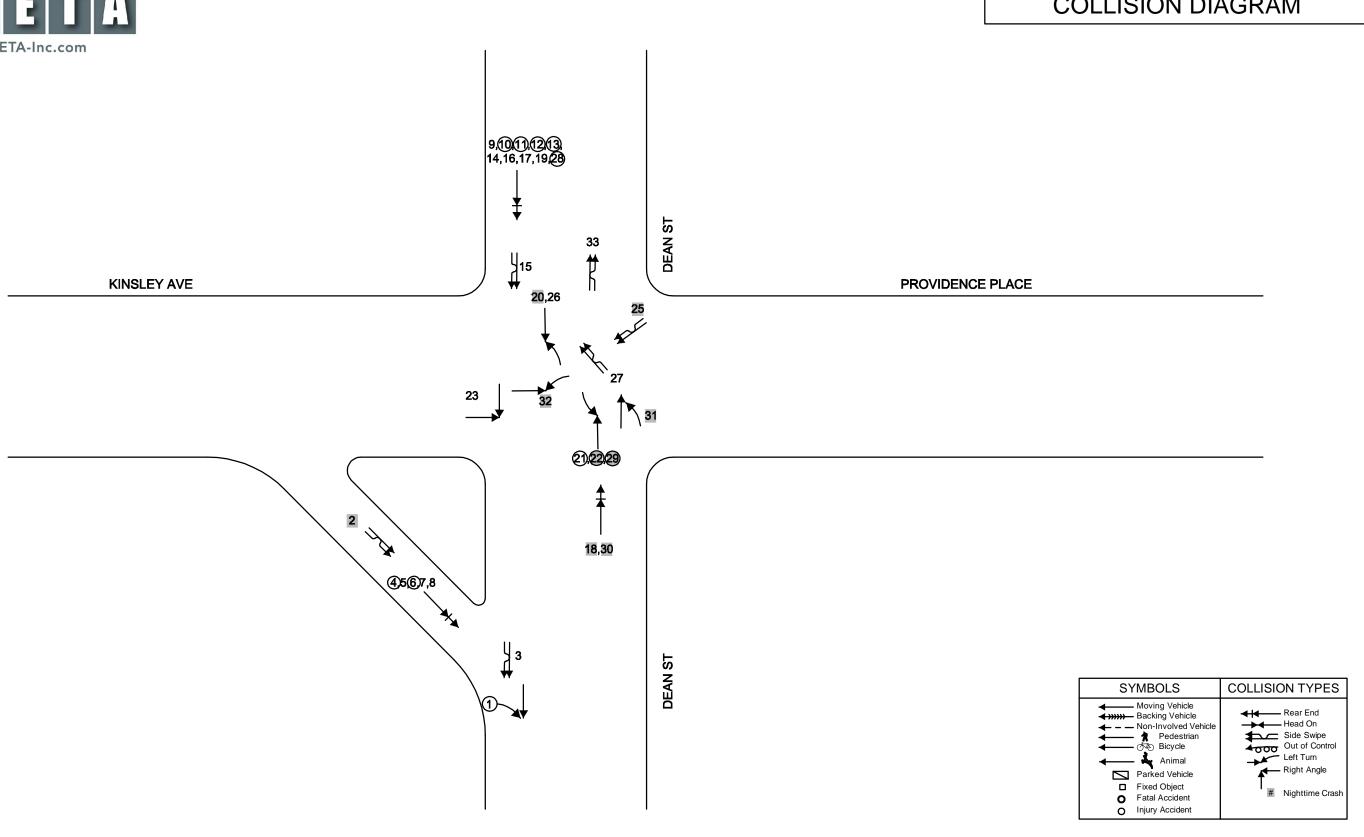
В

Crash Diagrams

Dean Street at Kinsley Avenue







Intersection: Dean Street at Kinsley Avenue Date Range: January 2017 - December 2018

Proposed	d Commercial	Redevelo	pment
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Appendix

Providence, Rhode Island

В

Crash Reports

Dean Street at Kinsley Avenue (Channelized EB Right Turn)



STATE OF RHODE ISLAND UNIFORM CRASH REPORT

Reporting Agency Name Report Number								Cr	ash Date		Cras	h Time	W	/alk In Re	port	Parkin	g Lot		
Providence 2017-00						00544	152		0	06/08/2017 2			12]	
	Town Name			Stre	et or Highw	ay				On Ramp	Exit	#	# of La	nes Pos	sted Spee	d Limit			
Provid	lence			DE	AN ST				1	Off Ramp			3		25	□ N//	ال ۵	Jnk	
Nearest	Intersection	n Street		Directio	n From Nea	rest Int	ersection	to Cras	h Site	Distance	From N	earest	Inter.	Lattitud	de	Long	jitude		
				✓ At I	nter. No	rth 🔲 S	South 🗌	East	West	0		Feet [✓ Miles	41.82	881	5000	42658	3	
Unit ID	Driver's Las	t Name		First Na	ime	M.I. DC)B	Uni	t ID	Driver's Las	st Name		F	irst Nam	ie	M.I. D	OB		
1									2										
Address	S				City			Add	dress						City				
					NEW BE	DFOR)								CRANS				
State	Zip	Home F	Phone	Cell Pho	one	Work F	Phone	Sta	ite Z	Zip	Home	Phone	e (Cell Phon	e	Work	Phone		
MA	02745							RI	(02920									
Driver's	License #	•			□CDL	Lic. St	ate	Dri	ver's l	License #			_	1	CDL	Lic. S	tate		
					ПСРГ	MA										RI			
M/V Vid	olation	M/V Vic	lation	M/V Vio	olation	M/V V	iolation	M/	V Viol	ation	M/V \	iolatio/	n	M/V Viola	tion	M/V \	/iolatior	1	
Driver	/Owner Same	Owner's	s Last Nam	ie	First Na	ame	V	1.I. D		Owner Same	Owne	r's Las	t Name		First N	lame		M.I.	
	Ш				Cib			۸d	dress	✓					City				
Addres	S				City			Adi	uress						City				
Chala	7:	Home I	Dhono	Cell Ph	WESTPO	Work	Phone	Sta	ate	Zip	Home	Phone	e (Cell Phon	e e	Work	Phone		
	Zip	nome	rione	Cell Fil	OHC	VVOIR	Hone												
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Hit And	d Bun							Hit	And	Run									
	, M/V & Dri	vor Left S	cene 🗖	Vec Drive	er Left Scene	e 🗸	No 🔲			M/V & Driv	er Left	Scene	☐ Yes	s, Driver	Left Scen	e 🗸	No [Unk	
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_	e Towed?	Towing	Company	Name		Haz Ma	at Placard	i? Ve	hicle '	Towed?	Towi	ng Cor	npany Na	any Name Haz Mat Placard?					
Yes	√ No					Yes	√ N	。	☐Yes ✓ No					☐Yes ✓ No					
							Pe	rson	Гуре	:									
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or N/A	,	7 8	9 16 Un	ık. Seat	20 Othe 21 Tow		osed Area	3 Front 4 Side	7 Ur	nk. 4 N/A 5 Unl		4 Shou 5 Lap (ilder Only Only	10 Child 11 Helm		4 Fa 5 No	tal Injury		
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Home	Phone	Ceil	rnone	l vvc	OIN FIIONE		Damage												
Dames								Donort	ina Of	fficer Badg	o Numb	er	Rep	ort Date					
	rting Officer ert Heaton	Name						539	ing Oi	nicer badg	e Mullic			/09/20:	17				

Last Mod: 6/3/2018 5:15 AM

Page 1

https://www.crashlogic.com

Report Number 2017-00054452

STATE OF RHODE ISLAND UNIFORM CRASH REPORT Narrative/Diagram Supplemental

On Thursday, June 08, 2017 at approximately 2212 hrs, Vehicle #1 was being operated in an easterly direction on Kinsley Ave at

the intersection with Dean St. Vehicle #1 then began to make a right turn from Kinsley Ave onto Dean St in a southerly direction. At this time the front right corner of Vehicle #1 came into collision with the left side of Vehicle #2, which was also making a right turn from Kinsley Ave onto Dean St in a southerly direction. There were no injuries reported and no disinterested witnesses approached police. rerbally reported to police that he was driving his vehicle in the right travel lane when Vehicle #2 The operator of Vehicle #1 tried to pass him on the right in the break down area on Dean St and it was at this time the left side of Vehicle #2 collided with the front left side o vehicle. ve<u>rbally repo</u>rted to police that he w<u>as driving</u> in the right travel lane while turning right onto The operator of Vehicle #2, it was at this time that Vehicle #1 drifted from on the left. According to Dean St when Vehicle #1 began to pass the left lane into the right lane and collided with vehicle. Vehicle #1 sustained damage to the front right fender, front bumper and was driven from the scene by the operator. Vehicle #2 sustained damage to the rear left quarter panel, rear left door, front left door, rocker panel and was driven from the scene by the operator. At the time of this investigation it was dark, the area was lit with artificial light, the weather was clear, the road surface was dry and free from defects and obstructions. The Pavement Markings was operating properly prior to the crash. This crash report was created on 6/9/2017 by Robert Heaton badge #539. {# - - - Do Not Delete - - - {*6/9/2017 1:36:40 AM*}#}

STATE OF RHODE ISLAND UNIFORM CRASH REPORT

Reporting Agency Name Re						ort Number				Cras	sh Date		Crash Time			Walk In Report			king Lot		
					2017-00015752									25		Walk III Kepoit			King Lot		
City or Town Name Street or Hig							3732			On Ramp Exit #					2000	Posted Speed Limit			Ш		
L					KINSLEY AVE						✓ Off Ramp					-					
					From Ne		ntersecti	on to C			Distance		aract I	25				∐N/A ∐Unk Longitude			
				100000000000000000000000000000000000000	iter. \square No																
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Reporting O	fficer Nar	ne							ting Offi	icer	Badge N	umber			rt Date						
Joseph Iar	ITIUCCI							410						11/1	1/201	7					

Report Number **2017-00015752**

STATE OF RHODE ISLAND UNIFORM CRASH REPORT Narrative/Diagram Supplemental

On Friday, November 03, 2017 at approximately 10:25 hrs., Vehicle #1 was operated in a easterly direction on KINSLEY AVE when, at the intersection of DEAN ST, the Front Driver Side(11) of the vehicle came into collision with the Front Passenger Side(2) of Vehicle #2, which was traveling south on DEAN ST and Movements Essentially Straight Ahead.

There were no other injuries reported and no disinterested witnesses approached police.

Vehicle #1 sustained damage to the Front Driver Side(11) and was driven from the scene by the operator.

Vehicle #2 sustained damage to the Front Passenger Side(1) and was driven from the scene by the operator.

At the time of this investigation the weather was Clear, the road surface was Dry and the light condition was Daylight. The Traffic Control Signal was operating properly prior to the crash.

This crash report was created on 11/11/2017 by Joseph Iannucci badge #410.

{# - - - Do Not Delete - - - {*11/11/2017 4:56:10 PM*}#}

STATE OF RHODE ISLAND UNIFORM CRASH REPORT

Reporting Agency Name					Report Num		C	Crush Bute			Cras	h Time	Wa	ılk In Rep	ort	Parking	Lot		
Providence					2017-00136026					12/29/2017			13:57						
	Town Name			Stree	et or Highwa	У				On R	amp	Exit #		# of Lane	es Post	ed Speed			.
Provid	ence			KIN	SLEY AVE					Off R						25	□ N/A		ık
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Report Number 2017-00136026

STATE OF RHODE ISLAND UNIFORM CRASH REPORT Narrative/Diagram Supplemental

On Friday, December 29, 2017 at approximately 13:57 hrs., Vehicle #1 was operated in an easterly direction on Kinsley Ave when, at approximately 420 Feet North of intersection of Dean St, the front of the vehicle came into collision with the rear of Vehicle #2, which was stopped in traffic facing East on Kinsley Ave.

The operator of Vehicle #1 stated to Police that she thought Vehicle #2 had moved as she was looking to her left for traffic when her vehicle collided with Vehicle #2.

The operator of Vehicle #2 stated to Police that she was stopped at the yield sign prior to the collision.

A query on the license (RI License #) to the operator of Vehicle #1, expired.

There were no other injuries reported and no disinterested witnesses approached police.

Vehicle #1 sustained damage to the front and was driven from the scene by the owner,

Vehicle #2 sustained damage to the rear and was driven from the scene by the operator.

The operator of Vehicle was given a court date of February 8, 2018 at the Rhode Island Sixth Division District Court for operating on an expired license.

At the time of this investigation the weather was clear, the road surface was dry and free of debris or obstructions. The light condition was daylight. The yield sign was operating properly prior to the crash.

This crash report was created on 12/29/2017 by Fernando Lopez badge #156.

STATE OF RHODE ISLAND UNIFORM CRASH REPORT

Reporting Agency Nam	ρ	Report Num	ber			Crash D	ate	Cra	sh Time	Wa	lk In Rep	ort	arking	Lot	
Providence	-	2018-00		06		02/2	7/201	8 07	7:33	; 🗆					
City or Town Name	Street or Highwa				On I	Ramp	Exit #	# of Lane	es Poste	ed Speed					
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Address		City			Addr					C	ity				
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Driver's License #		□CDL	RI							L]CDL	RI			
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Address		City									PROVID	ENCE			
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Owner			Add	dress											
	Call Phone	Work Phone		Damage	Descript	ion									
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Reporting Officer Na	ame					ng Office	r Badge	Number		ort Date 27/201	18				
Fernando Lopez					156				02/	2, 120.				la ele c	

Report Number 2018-00020696

STATE OF RHODE ISLAND UNIFORM CRASH REPORT Narrative/Diagram Supplemental

On Tuesday, February 27, 2018 at approximately 07:33 hrs., Vehicle #1 was operated in an easterly direction on Kinsley Ave when, at approximately 339 Feet North of intersection of Dean St, the front of the vehicle came into collision with the rear of Vehicle #2, which was stopped in traffic facing East on Kinsley Ave.

The operator of Vehicle #1 stated to Police that he was behind Vehicle #2 looked to her left for other vehicles and couldn't stop in time to avoid the collision.

The operator of Vehicle #2 stated to Police that he was stopped at the yield sign prior to Vehicle #1 colliding with her vehicle.

The operator of Vehicle #2, DOB disinterested witnesses approached police.

Vehicle #1 sustained damage to the front and was towed from the scene by All City Towing.

Vehicle #2 sustained damage to the rear and was driven from the scene by the operator.

The operator of Vehicle #1, (DOB COLOR), was issued Universal Traffic Summons # for operating a motor vehicle when registration is suspended and assigned a court date of March 27, 2018 at the Rhode Island Traffic Tribunal (RITT).

Police seized the plates to Vehicle #1 and turned them over to the Rhode Island Department of Motor Vehicles (RI DMV).

At the time of this investigation the weather was clear, the road surface was wet and free of debris or obstructions. The light condition was daylight. The yield signs were properly displayed prior to the crash.

This crash report was created on 2/27/2018 by Fernando Lopez badge #156.

STATE OF RHODE ISLAND UNIFORM CRASH REPORT

Reporting Agency N	lame	Report Nu	mber			Cra	ash Date		Crash Time		Walk In R	eport	Parkin	g Lot
Providence		2018-0		350			3/16/20	18	08:48			.		j
City or Town Name		Street or Highw				_	On Ramp	Exit #		Lanes Po	sted Spe	ed Limit		
Providence		KINSLEY AV				_	Off Ramp				25	□ N//	A П	Jnk
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		☐ At Inter. ✓ No					265		et Mile	s 41.8 2	2843	-71	4265	3
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Driver's License #			Lic. Sta	ate			icense #					Lic. S	tate	
		□CDL	MA								CDL	RI		
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2 Passenger 3 Pedestrian	5 Other Cyclist 6 Witness	Conveyance, etc 8 Occupant of M		n. not in	Transpo	rt (Par	ked, etc.)		Unknown	Type of N	OII-MOCOI	isc		
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2 Unit 2 F Female 3 (etc.) U Unk.	1 2 3 14 Unk. 4 5 6 15 Othe		per er Enclose	d Area	1 N/A 2 No	5 Oth 6 Con			None Used Shoulder & La		- Rear Faci er Seat		n-incapa capacitat	acitating ing
or N/A	7 8 9 16 Unk	Seat 20 Othe 21 Tow	er Unenclo	sed Area	3 Front 4 Side	7 Unk			Shoulder Only ap Only	10 Child 11 Helm		4 Fat	tal Injury	
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Names Comments	Witnesses Dodestr	ione Diculists	Person	Unit ID	Sex		DOB	Seat Pos.	Air Bag Deployed	d Fiecter	Prot. System	Injury		ns by scue
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Adam Chin					289				03	3/16/20:	18			

Report Number **2018-00026850**

STATE OF RHODE ISLAND UNIFORM CRASH REPORT Narrative/Diagram Supplemental

On Friday, March 16, 2018 at approximately 08:48 hrs., Vehicle #1 was operated in a southerly direction on Kinsley Ave. when, at approximately 265 feet north of intersection of Dean St., the front(12) of the vehicle came into collision with the rear(6) of Vehicle #2, which was traveling south on Kinsley Ave. and movements essentially straight ahead. The operator of Vehicle #1 stated that he was merging on to Dean St. from Kinsley Ave. and as he was glancing back to assure that the lane of travel was clear, the front(12) of his vehicle struck the rear(6) of Vehicle #2. The operator of Vehicle #2 stated that he was awaiting to merge on to Dean St., when he was struck by the front(12) of Vehicle #1.

There were no other injuries reported and no disinterested witnesses approached police.

Vehicle #1 sustained no visible damage to the front(12) and was driven from the scene by the operator.

Vehicle #2 sustained damage to the rear(6) and was driven from the scene by the operator.

At the time of this investigation the weather was clear, the road surface was dry and the light condition was daylight.

This crash report was created on 3/16/2018 by Adam Chin badge #289.

{# - - - Do Not Delete - - - {*3/16/2018 10:01:03 AM*}#}

STATE OF RHODE ISLAND UNIFORM CRASH REPORT

	A			Panar	t Numb	ner			Crash	Date	Cra	sh Time	Wal	k In Repo	ort Pa	arking L	.ot
	Agency Na	ine		1 ')2793	0		1	19/20:	18 08	3:47					
Provide	ence own Name			Street or H						Ramp	Exit #	# of Lane	es Poste	d Speed	Limit		
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11-14 10 0	vissorle I act	Namo		First Name		I. DOB				er's Last	Name	Fir	st Name	N	1.I. DOE	3	
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Page 1

Report Number 2018-00027930

approached police.

STATE OF RHODE ISLAND UNIFORM CRASH REPORT Narrative/Diagram Supplemental

On Monday, March 19, 2018 at approximately 08:47 hrs., Vehicle #1 was operated in an easterly direction on Kinsley Ave when, at approximately 508 Feet North of intersection of Dean St, the front of the vehicle came into collision with the rear of Vehicle #2,

which was stopped in traffic facing East on Kinsley Ave.

The operator of Vehicle #1 stated to Police that he was behind Vehicle #2 prior to the collision. The operator of Vehicle #2 stated to Police that she was stopped at the yield sign when Vehicle #1 collided with her vehicle. DOB suffered injuries and was transported to Rhode Island Hospital The operator of Vehicle #2. (RIH) by Providence Fire Rescue #4 with Lt Mello in charge. There were no other injuries reported and no disinterested witnesses

Vehicle #1 sustained damage to the front and was driven from the scene by the operator.

Vehicle #2 sustained damage to the rear and was towed from the scene by National Towing.

At the time of this investigation the weather was clear, the road surface was dry and free of debris or obstructions. The light condition was daylight. The yield sign was properly displayed prior to the crash.

This crash report was created on 3/19/2018 by Fernando Lopez badge #156.

STATE OF RHODE ISLAND UNIFORM CRASH REPORT

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	Reporting Officer Noel Field	06/24/2019													

Report Number **2018-00064692**

STATE OF RHODE ISLAND UNIFORM CRASH REPORT Narrative/Diagram Supplemental

On Sunday, June 24, 2018, at approximately 1127 hours, Vehicle #1 was operated in a southerly direction turning from Kinsley Ave onto Dean St when, at the intersection of Dean St, the front of Vehicle #1 came into collision with the rear of Vehicle #2. Vehicle #2 was traveling from Kinsley Ave east and negotiating a curve turning/ merging onto Dean St south in the same way as Vehicle #1.

There were no injuries reported at the time of this investigation.

No disinterested witnesses approached police at the scene.

Vehicle #1 sustained damage to the front bumper assembly and was driven from the scene by the operator.

Vehicle #2 sustained damage to the rear bumper assembly and was driven from the scene by the operator.

At the time of this investigation the weather was cloudy, the road surface was dry and the light condition was daylight.

This crash report was created on 6/24/2018 by Noel Field FID #260.

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APPENDIX C – Trip Generation

ITE Trip Generation Summary

Site Trip Distribution

Build Conditions

Build Alternative (Woonasquatucket River Greenway Extension)

ITE Land Use Code

ITE Land Use Code 151 – Mini-Warehouse

ITE Lane Use Code 820 – Shopping Center

ITE Land Use Code 960 – Super Convenience Market/Gas Station



Pro	nosed	Commer	rial Red	eveloi	nment
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Appendix

Providence, Rhode Island

ITE Trip Generation Summary

Trip Generation Summary

Summary;

Summary;	<u>Description</u>	Enter	Exit	Total
AM Peak Hour				
ITE Land Use Code 151	Mini-Warehouse	6	5	11
ITE Land Use Code 820	Shopping Center	4	2	6
ITE Land Use Code 960	Super Convience Market/Gas Station	225	225	450
	TOTAL	235	232	467
PM Peak Hour				
ITE Land Use Code 151	Mini-Warehouse	8	8	16
ITE Land Use Code 820	Shopping Center	12	13	25
ITE Land Use Code 960	Super Convience Market/Gas Station	185	184	369
	TOTAL	205	205	410



Calculations;

ITE Land Use Code 151 Mini-Warehouse

(805 Units)

Independent Variable (X) = Hundred Units

X = 8.05

AM Peak

Directional Distribution:

51% Entering 49% Exitina

1.39 x (X) Т 1.39 x 8.05 11

Enter-6 Exit: 5 Total:

PM Peak

Directional Distribution:

50% Entering

1.95 x (X) 1.95 x 8.05 16

Enter: 8 Exit: 8 Total: 16

ITE Land Use Code 820 Shopping Center

(6,500 SF)

Independent Variable (X) = Thousand Gross Floor Area

X = 6.5

AM Peak

Directional Distribution:

62% Entering

38% Exitina

50% Exiting

Т

0.94 x (X) 0.94 x 6.5 6

Exit: Total: 4 2

PM Peak

Directional Distribution:

48% Entering

52% Exiting

Enter-

Т 3.81 x (X) Т 3.81 x 6.5 25

Enter-12 Exit: 13 Total:



Total: 369

ITE Land Use Code 960 S	(16 VFP)		
Independent Variab	le (X) = Vehicle Fueling Positions (VFP)	X = 16	
AM Peak	Directional Distribution:	50% Entering	50% Exiting
	T = 28.08 x (X) $T = 28.08 x 16$ $T = 450$	Enter: Exit: Total:	225 225 450
PM Peak	Directional Distribution:	50% Entering	50% Exiting
	$T = 22.96 \times (X)$ $T = 22.96 \times 16$	Enter: Exit:	185 184

T = 369



Pro	oosed	Commercial	Redevelo	pment

Appendix

Providence, Rhode Island

C

Site Trip Distribution

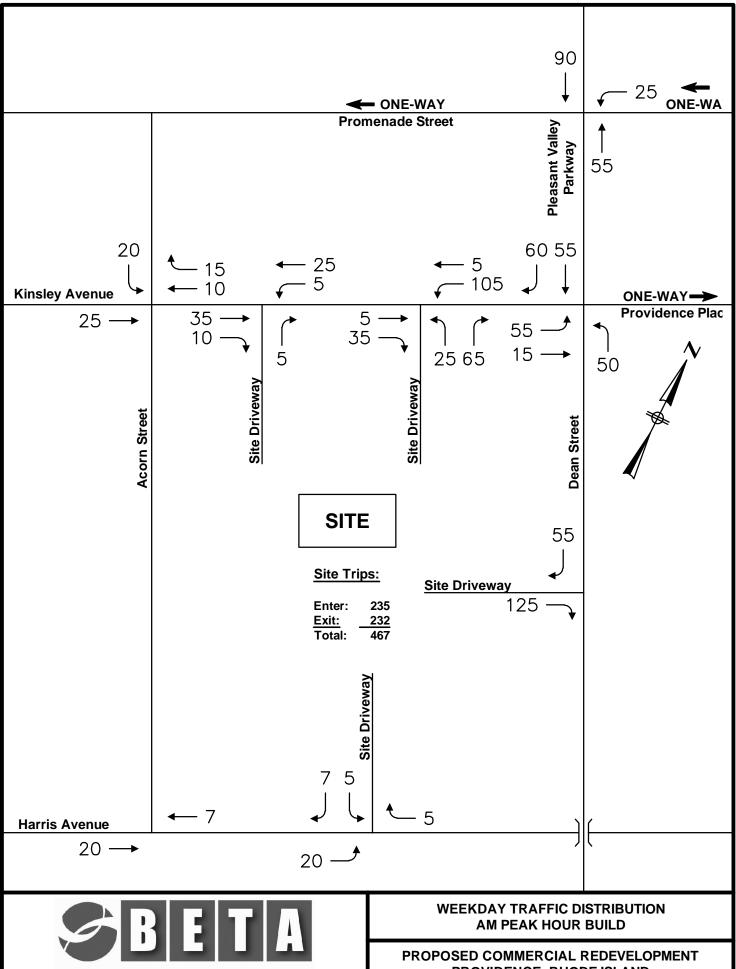
Build Conditions

Build Alternative (Woonasquatucket River Greenway Extension)



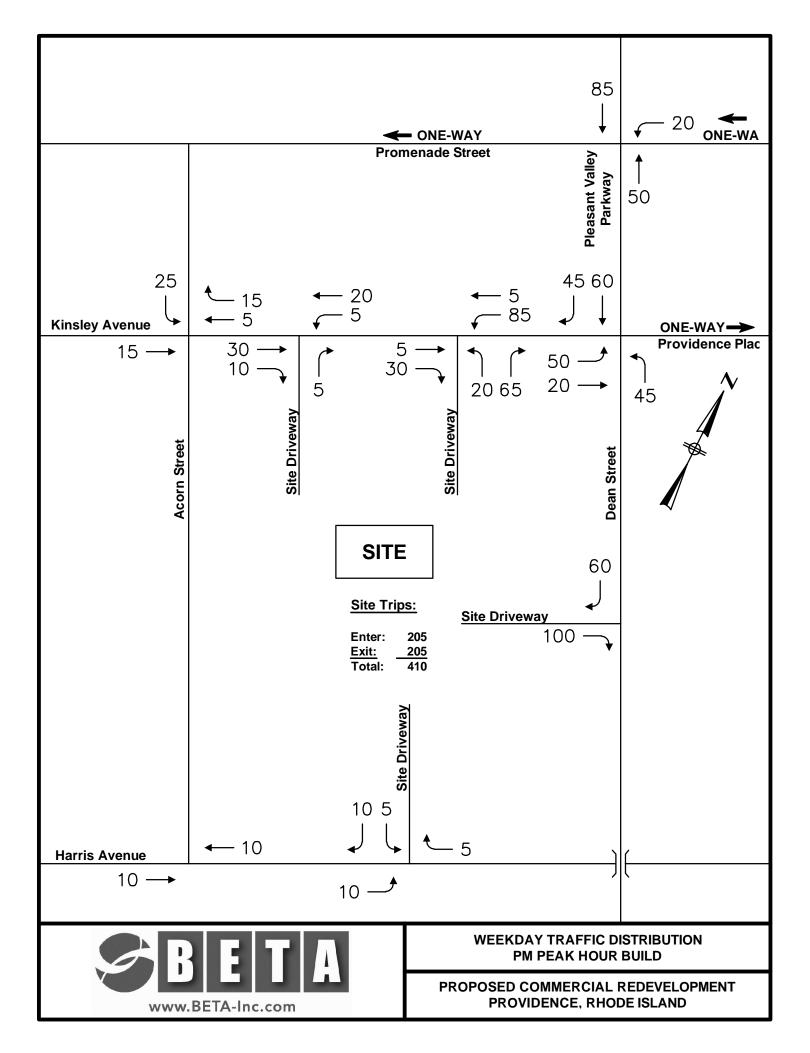
Build Conditions





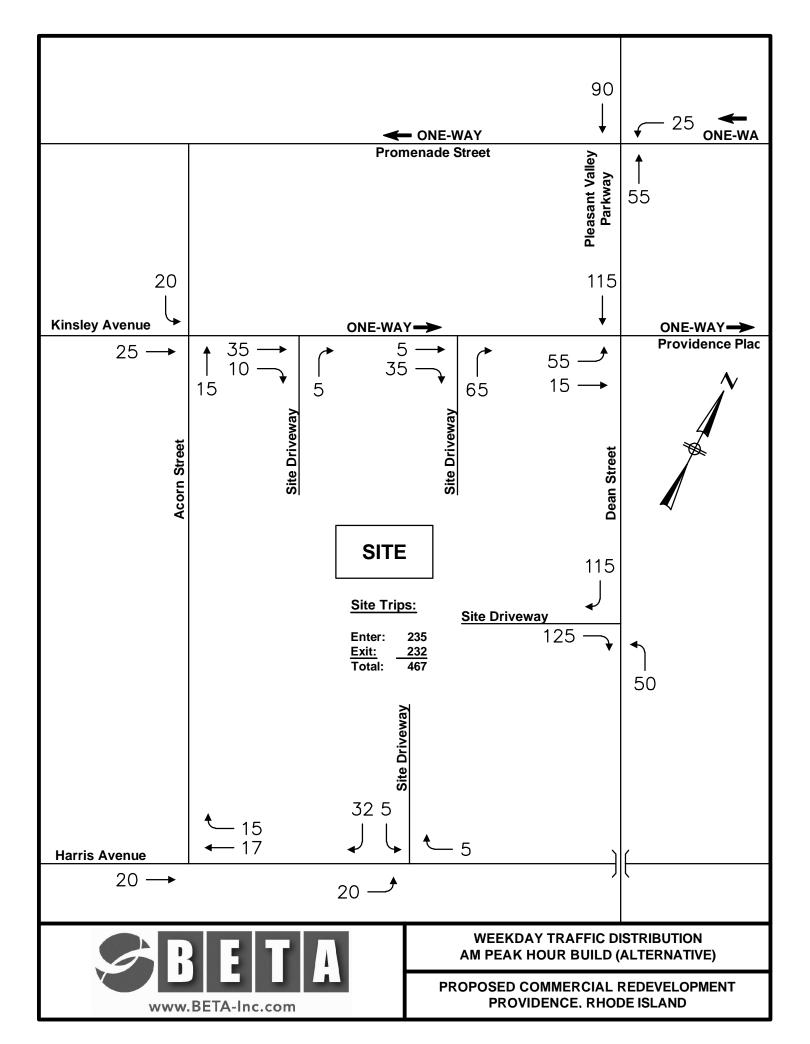


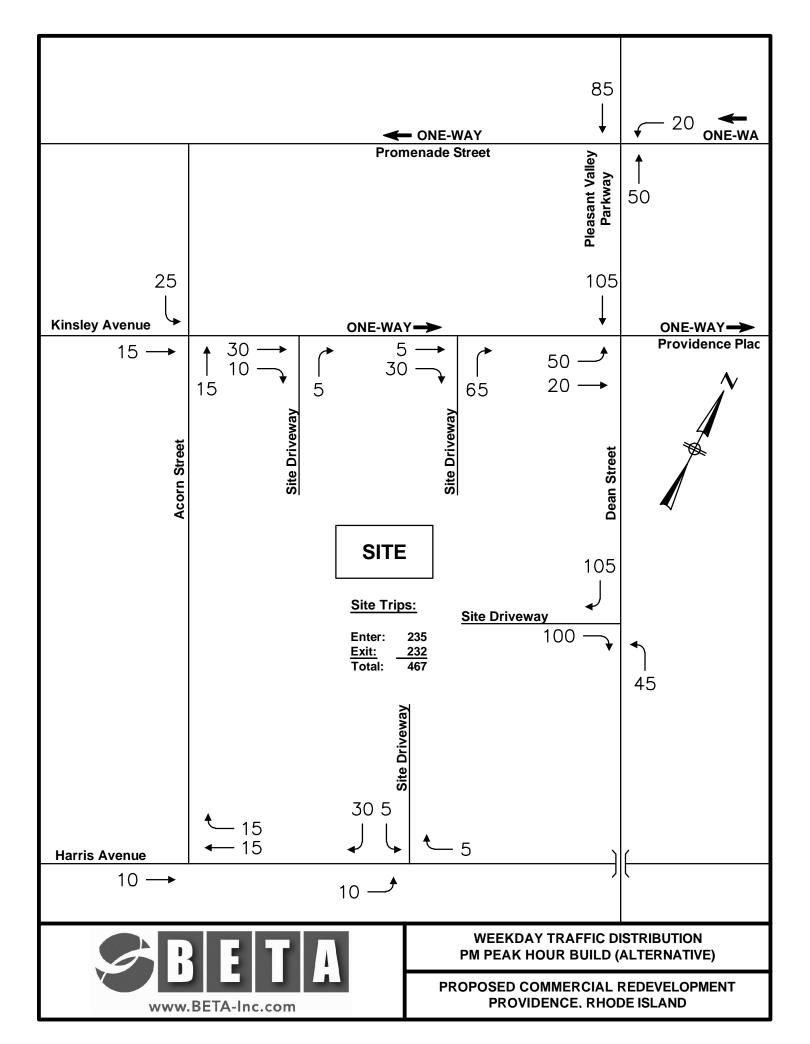
PROVIDENCE, RHODE ISLAND



Build Alternative (Woonasquatucket River Greenway Extension)







Proposed	Commercial	Redevel	onment
rioposeu	Commercial	Neucvei	Opinent

Appendix

Providence, Rhode Island

C

ITE Land Use Code

ITE Land Use Code 151 – Mini-Warehouse

ITE Lane Use Code 820 – Shopping Center

ITE Land Use Code 960 – Super Convenience Market/Gas Station



ITE Land Use Code 151 – Mini-Warehouse



Land Use: 151 Mini-Warehouse

Description

A mini-warehouse is a building in which a number of storage units or vaults are rented for the storage of goods. They are typically referred to as "self-storage" facilities. Each unit is physically separated from other units, and access is usually provided through an overhead door or other common access point.

Additional Data

Time-of-day distribution data for this land use are presented in Appendix A. For the 10 general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 10:30 and 11:30 a.m. and 1:15 and 2:15 p.m., respectively.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in California, Colorado, Massachusetts, Minnesota, New Jersey, Texas, and Utah.

Source Numbers

212, 403, 551, 568, 642, 708, 724, 850, 868, 876



Mini-Warehouse

(151)

Vehicle Trip Ends vs: Storage Units (100s)

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

One Hour Between 7 and 9 a.

Setting/Location: General Urban/Suburban

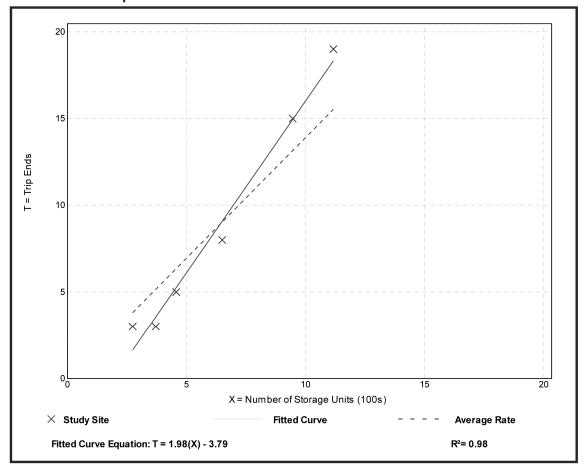
Number of Studies: 6 Avg. Num. of Storage Units (100s): 6

Directional Distribution: 51% entering, 49% exiting

Vehicle Trip Generation per Storage Unit (100s)

Average Rate	Range of Rates	Standard Deviation
1.39	0.81 - 1.70	0.33

Data Plot and Equation





Mini-Warehouse

(151)

Vehicle Trip Ends vs: Storage Units (100s)

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

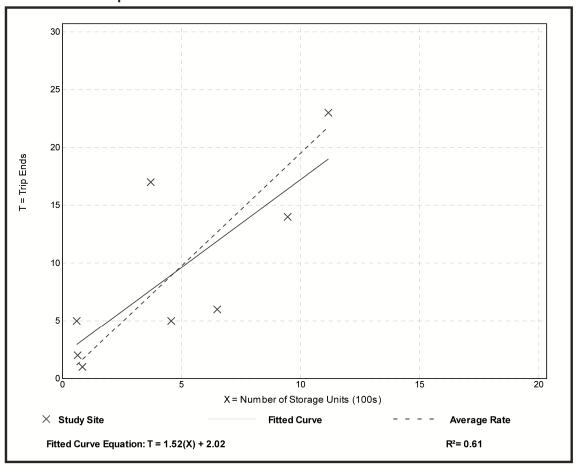
Number of Studies: 8
Avg. Num. of Storage Units (100s): 5

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Storage Unit (100s)

Average Rate	Range of Rates	Standard Deviation
1.95	0.92 - 8.33	1.40

Data Plot and Equation





ITE Lane Use Code 820 – Shopping Center



Land Use: 820 Shopping Center

Description

A shopping center is an integrated group of commercial establishments that is planned, developed, owned, and managed as a unit. A shopping center's composition is related to its market area in terms of size, location, and type of store. A shopping center also provides on-site parking facilities sufficient to serve its own parking demands. Factory outlet center (Land Use 823) is a related use.

Additional Data

Shopping centers, including neighborhood centers, community centers, regional centers, and super regional centers, were surveyed for this land use. Some of these centers contained non-merchandising facilities, such as office buildings, movie theaters, restaurants, post offices, banks, health clubs, and recreational facilities (for example, ice skating rinks or indoor miniature golf courses).

Many shopping centers, in addition to the integrated unit of shops in one building or enclosed around a mall, include outparcels (peripheral buildings or pads located on the perimeter of the center adjacent to the streets and major access points). These buildings are typically drive-in banks, retail stores, restaurants, or small offices. Although the data herein do not indicate which of the centers studied included peripheral buildings, it can be assumed that some of the data show their effect.

The vehicle trips generated at a shopping center are based upon the total GLA of the center. In cases of smaller centers without an enclosed mall or peripheral buildings, the GLA could be the same as the gross floor area of the building.

Time-of-day distribution data for this land use are presented in Appendix A. For the 10 general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 11:45 a.m. and 12:45 p.m. and 12:15 and 1:15 p.m., respectively.

The average numbers of person trips per vehicle trip at the 27 general urban/suburban sites at which both person trip and vehicle trip data were collected were as follows:

- · 1.31 during Weekday, AM Peak Hour of Generator
- 1.43 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 4 and 6 p.m.
- 1.46 during Weekday, PM Peak Hour of Generator

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), British Columbia (CAN), California, Colorado, Connecticut, Delaware, District of Columbia, Florida, Georgia, Illinois, Indiana, Iowa, Kansas, Kentucky, Maine, Maryland, Massachusetts, Michigan, Minnesota, Nevada, New Jersey, New York, North Carolina, Ohio, Oklahoma, Oregon, Pennsylvania, South Dakota, Tennessee, Texas, Vermont, Virginia, Washington, West Virginia, and Wisconsin.

Source Numbers

105, 110, 154, 156, 159, 186, 190, 198, 199, 202, 204, 211, 213, 239, 251, 259, 260, 269, 294, 295, 299, 300, 301, 304, 305, 307, 308, 309, 310, 311, 314, 315, 316, 317, 319, 358, 365, 376, 385, 390, 400, 404, 414, 420, 423, 428, 437, 440, 442, 444, 446, 507, 562, 580, 598, 629, 658, 702, 715, 728, 868, 870, 871, 880, 899, 908, 912, 915, 926, 936, 944, 946, 960, 961, 962, 973, 974, 978



Shopping Center (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

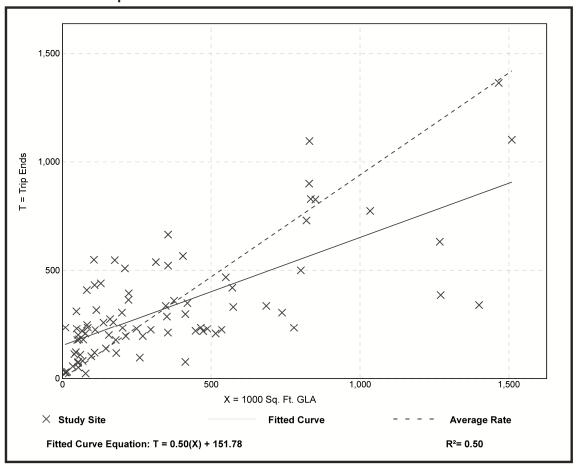
Number of Studies: 1000 Sq. Ft. GLA: 351

Directional Distribution: 62% entering, 38% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
0.94	0.18 - 23.74	0.87

Data Plot and Equation





Shopping Center (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

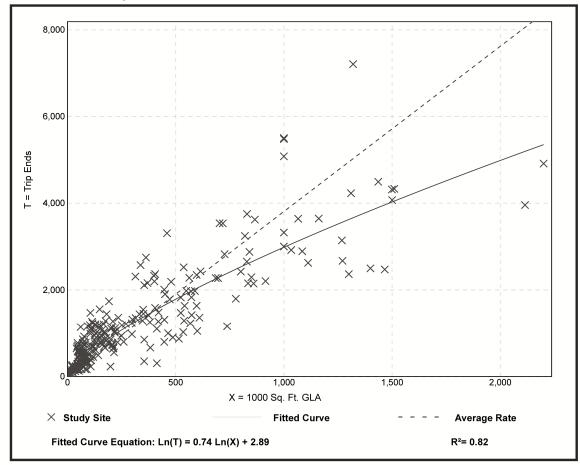
Number of Studies: 1000 Sq. Ft. GLA: 327

Directional Distribution: 48% entering, 52% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
3.81	0.74 - 18.69	2.04

Data Plot and Equation





ITE Land Use Code 960 – Super Convenience Market/Gas Station



Land Use: 960 Super Convenience Market/Gas Station

Description

This land use includes gasoline/service stations with convenience markets where there is significant business related to the sale of convenience items and the fueling of motor vehicles. Some commonly sold convenience items include newspapers, freshly brewed coffee, daily-made donuts, bakery items, hot and cold beverages, breakfast items, dairy items, fresh fruits, soups, light meals, ready-to-go and freshly made sandwiches and wraps, and ready-to-go salads. Stores typically also had automated teller machines (ATMs), and public restrooms. The sites included in this land use category have the following two specific characteristics:

- The gross floor area of the convenience market is at least 3,000 gross square feet
- The number of vehicle fueling positions is at least 10

Convenience market with gasoline pumps (Land Use 853) and gasoline/service station with convenience market (Land Use 945) are related uses.

Additional Data

To reflect changing characteristics of the convenience market component of this land use, only data from the past two decades have been included in this land use.

The independent variable, vehicle fueling positions, is defined as the maximum number of vehicles that can be fueled simultaneously. Gasoline/service stations in this land use include "pay-at-the-pump" and traditional fueling stations.

A multi-variable regression analysis based on both the convenience market gross floor area (GFA) and the number of vehicle fueling positions (VFP) produced a series of fitted curve equations. The equations are in the form of:

Vehicle Trips = [(VFP Factor) x (Number of VFP)] + [(GFA Factor) x (GFA)] + (Constant)

The values for the VFP factor, GFA factor, and constant are presented in the following table for each time period for which a fitted curve equation could produce an R² value of at least 0.50.

Time Period	VFP Factor	GFA Factor	Constant	R²
Weekday, AM Peak Hour of Generator	10.3	105	-290	0.62
Weekday, PM Peak Hour of Generator	6.91	76.0	-133	0.68
Weekday, AM Peak Hour of Adjacent Street	16.1	135	-483	0.66
Weekday, PM Peak Hour of Adjacent Street	11.5	82.9	-226	0.51

The sites were surveyed in the late 1990's, 2000s and the 2010s in Florida, Iowa, Maryland, Minnesota, New Hampshire, New Jersey, Pennsylvania, Texas, Utah, and Wisconsin.

Source Numbers

617, 813, 844, 850, 864, 865, 867, 869, 882, 888, 904, 938, 954, 960, 962



Super Convenience Market/Gas Station

(960)

Vehicle Trip Ends vs: Vehicle Fueling Positions

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 39

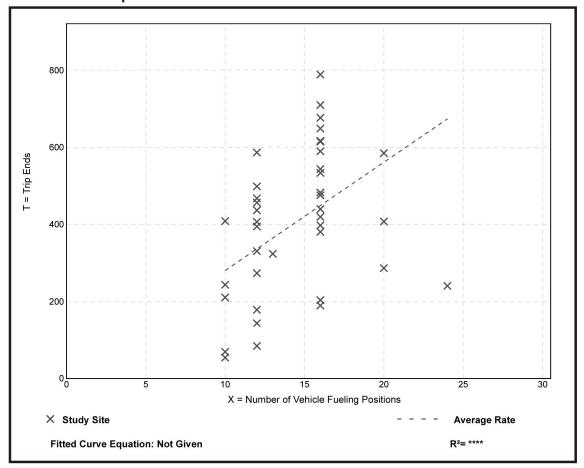
Avg. Num. of Vehicle Fueling Positions: 14

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
28.08	5.40 - 49.31	11.98

Data Plot and Equation





Super Convenience Market/Gas Station (960)

Vehicle Trip Ends vs: Vehicle Fueling Positions

Weekday, On a:

> Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

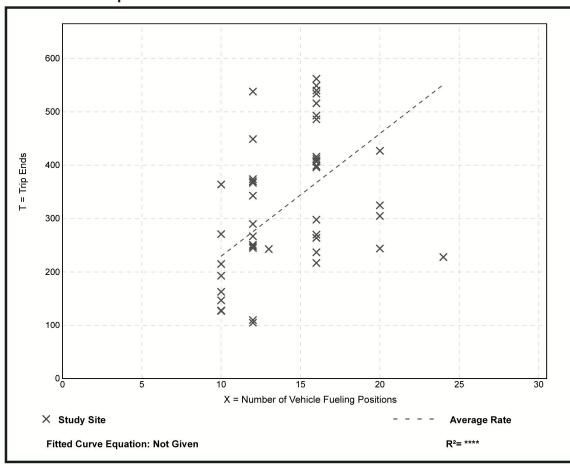
Number of Studies: Avg. Num. of Vehicle Fueling Positions: 14

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
22.96	8.75 - 44.83	8.34

Data Plot and Equation





APPENDIX D - Operational Analysis

Existing Conditions

Dean Street at Kinsley Avenue/Providence Place Pleasant Valley Parkway at Promenade Street Kinsley Avenue at Acorn Street

Future No-Build Conditions

Dean Street at Kinsley Avenue/Providence Place Pleasant Valley Parkway at Promenade Street Kinsley Avenue at Acorn Street

Future Build Conditions

Dean Street at Kinsley Avenue/Providence Place Pleasant Valley Parkway at Promenade Street Kinsley Avenue at Acorn Street Kinsley Avenue at Western Site Driveway Kinsley Avenue at Eastern Site Driveway

Future Build Alternative

Dean Street at Kinsley Avenue/Providence Place Pleasant Valley Parkway at Promenade Street Kinsley Avenue at Acorn Street Dean Street at Site Driveway Kinsley Avenue at Western Site Driveway Kinsley Avenue at Eastern Site Driveway



Proposed	d Commercial	Redevelo	pment
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Appendix

Providence, Rhode Island

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Existing Weekday AM / PM Peak Hour

Dean Street at Kinsley Avenue/Providence Place
Pleasant Valley Parkway at Promenade Street
Kinsley Avenue at Acorn Street

Dean Street at Kinsley Avenue/Providence Place





Turning Movement Diagram

Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

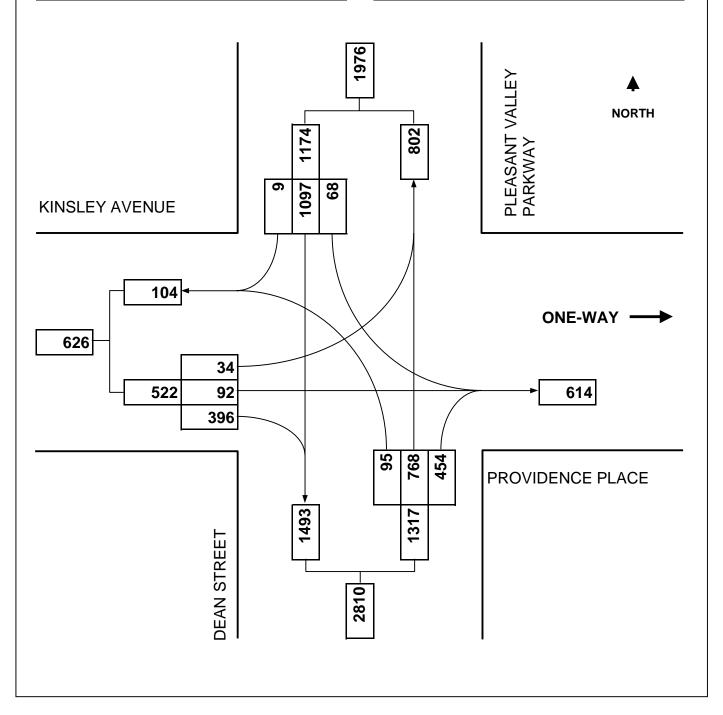
Existing: AM Peak Hour

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: 8:00 AM - 9:00 AM

Future: n/a



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7				7	↑ ↑		7	↑ ↑	
Traffic Volume (vph)	34	92	396	0	0	0	95	768	454	68	1097	9
Future Volume (vph)	34	92	396	0	0	0	95	768	454	68	1097	9
Satd. Flow (prot)	0	1875	1599	0	0	0	1805	3366	0	1805	3536	0
Flt Permitted		0.987					0.100			0.104		
Satd. Flow (perm)	0	1875	1599	0	0	0	190	3366	0	198	3536	0
Satd. Flow (RTOR)			165					173			3	
Lane Group Flow (vph)	0	131	413	0	0	0	98	1260	0	69	1128	0
Turn Type	Split	NA	Perm				Perm	NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2				1			1		
Total Split (s)	15.0	15.0	15.0				45.0	45.0		30.0		
Total Lost Time (s)		5.0	5.0				5.0	5.0		5.0		
Act Effct Green (s)		10.0	10.0				40.1	40.1		60.0	65.1	
Actuated g/C Ratio		0.12	0.12				0.47	0.47		0.71	0.76	
v/c Ratio		0.60	1.24				1.10	0.75		0.13	0.42	
Control Delay		49.2	153.3				157.0	19.8		6.5	1.1	
Queue Delay		0.0	0.0				0.0	0.0		0.4	0.3	
Total Delay		49.2	153.3				157.0	19.8		6.9	1.4	
LOS		D	F				F	В		Α	Α	
Approach Delay		128.2						29.7			1.7	
Approach LOS		F						С			Α	
Queue Length 50th (ft)		68	~195				~61	244		2	0	
Queue Length 95th (ft)		#144	#393				#166	362		m12	0	
Internal Link Dist (ft)		488			721			853			84	
Turn Bay Length (ft)			100				175			100		
Base Capacity (vph)		220	333				89	1677		620	2662	
Starvation Cap Reductn		0	0				0	0		315	829	
Spillback Cap Reductn		0	0				0	0		0	0	
Storage Cap Reductn		0	0				0	0		0	0	
Reduced v/c Ratio		0.60	1.24				1.10	0.75		0.23	0.62	

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 85.1

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.24

Intersection Signal Delay: 36.2

Intersection LOS: D ICU Level of Service B

Intersection Capacity Utilization 63.5%

Analysis Period (min) 15

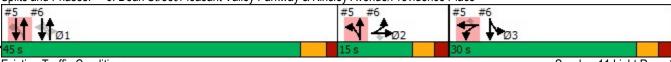
 Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Dean Street/Pleasant Valley Parkway & Kinsley Avenue/Providence Place



Existing Traffic Conditions Timing Plan: AM Peak Hour Synchro 11 Light Report



Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

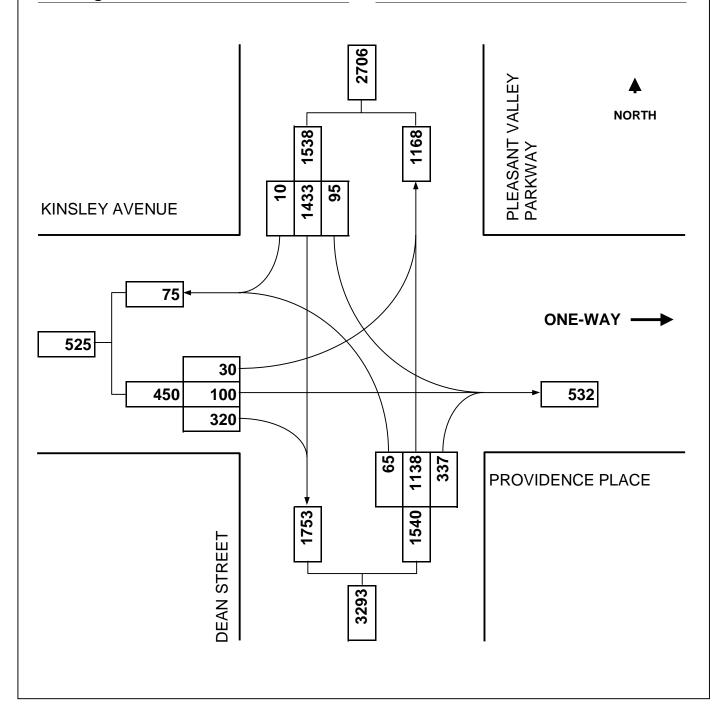
Existing: PM Peak Hour

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: 4:30 PM - 5:30 PM

Future: n/a



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ન	7				7	↑ ↑		7	↑ ↑	
Traffic Volume (vph)	30	100	320	0	0	0	65	1138	337	95	1433	10
Future Volume (vph)	30	100	320	0	0	0	65	1138	337	95	1433	10
Satd. Flow (prot)	0	1879	1599	0	0	0	1805	3434	0	1805	3536	0
Flt Permitted		0.989					0.114			0.114		
Satd. Flow (perm)	0	1879	1599	0	0	0	217	3434	0	217	3536	0
Satd. Flow (RTOR)			90					50			2	
Lane Group Flow (vph)	0	135	333	0	0	0	67	1520	0	97	1472	0
Turn Type	Split	NA	Perm				Perm	NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2				1			1		
Total Split (s)	15.0	15.0	15.0				40.0	40.0		35.0		
Total Lost Time (s)		5.0	5.0				5.0	5.0		5.0		
Act Effct Green (s)		10.0	10.0				35.1	35.1		63.1	68.2	
Actuated g/C Ratio		0.11	0.11				0.40	0.40		0.72	0.77	
v/c Ratio		0.63	1.28				0.78	1.09		0.15	0.54	
Control Delay		52.8	177.7				81.4	79.2		3.7	1.0	
Queue Delay		1.3	0.0				0.0	0.7		1.0	0.4	
Total Delay		54.1	177.7				81.4	79.9		4.7	1.4	
LOS		D	F				F	Е		Α	Α	
Approach Delay		142.0						79.9			1.6	
Approach LOS		F						Е			Α	
Queue Length 50th (ft)		75	~196				33	~518		2	0	
Queue Length 95th (ft)		#150	#365				#113	#655		m8	0	
Internal Link Dist (ft)		488			721			853			84	
Turn Bay Length (ft)			100				175			100		
Base Capacity (vph)		213	261				86	1396		699	2732	
Starvation Cap Reductn		0	0				0	0		433	628	
Spillback Cap Reductn		15	0				0	2		0	0	
Storage Cap Reductn		0	0				0	0		0	0	
Reduced v/c Ratio		0.68	1.28				0.78	1.09		0.36	0.70	

Cycle Length: 90

Actuated Cycle Length: 88.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.28

Intersection Signal Delay: 54.0
Intersection Capacity Utilization 68.1%

Intersection LOS: D
ICU Level of Service C

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

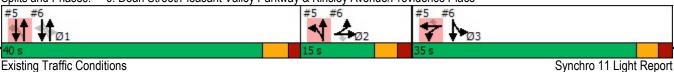
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Dean Street/Pleasant Valley Parkway & Kinsley Avenue/Providence Place



Timing Plan: PM Peak Hour

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Pleasant Valley Parkway at Promenade Street





Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

Reference No.: 5999

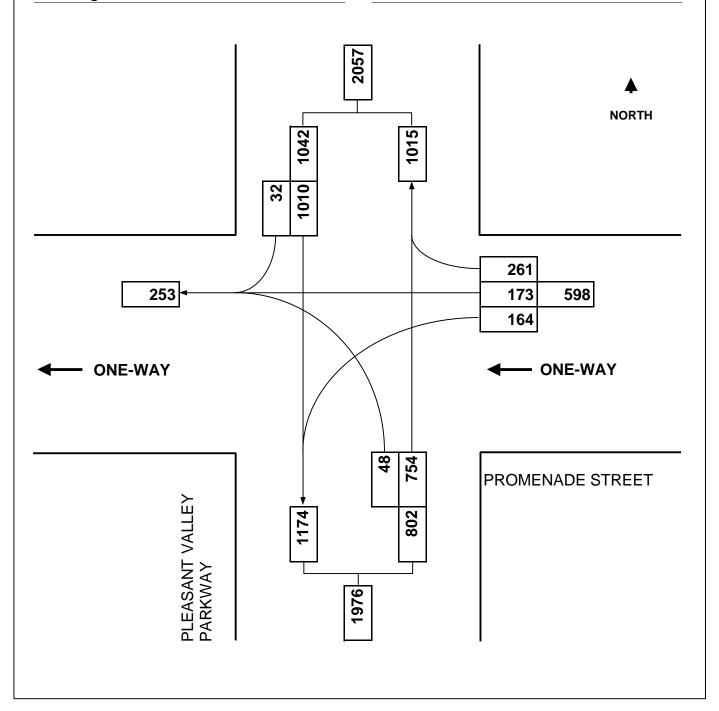
Existing: AM Peak Hour

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: 8:00 AM - 9:00 AM

Future: n/a



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				7	र्स	7		414			^	7
Traffic Volume (vph)	0	0	0	164	173	261	48	754	0	0	1010	32
Future Volume (vph)	0	0	0	164	173	261	48	754	0	0	1010	32
Satd. Flow (prot)	0	0	0	1698	1796	1615	0	3533	0	0	3574	1615
Flt Permitted				0.950	0.996			0.844				
Satd. Flow (perm)	0	0	0	1698	1796	1615	0	2991	0	0	3574	1615
Satd. Flow (RTOR)						182						85
Lane Group Flow (vph)	0	0	0	152	195	269	0	810	0	0	1041	33
Turn Type				Split	NA	Perm	D.P+P	NA			NA	Perm
Protected Phases				3	3		2	12			1	
Permitted Phases						3	1					1
Total Split (s)				30.0	30.0	30.0	15.0				45.0	45.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				19.9	19.9	19.9		50.1			40.1	40.1
Actuated g/C Ratio				0.23	0.23	0.23		0.59			0.47	0.47
v/c Ratio				0.38	0.46	0.52		0.44			0.62	0.04
Control Delay				30.0	31.5	13.4		1.7			19.5	0.1
Queue Delay				0.0	0.0	0.0		0.4			0.0	0.0
Total Delay				30.0	31.5	13.4		2.0			19.5	0.1
LOS				С	С	В		Α			В	Α
Approach Delay					23.2			2.0			18.9	
Approach LOS					С			Α			В	
Queue Length 50th (ft)				71	94	38		10			211	0
Queue Length 95th (ft)				127	158	106		16			305	0
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				500	528	603		1825			1683	805
Starvation Cap Reductn				0	0	0		469			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				0.30	0.37	0.45		0.60			0.62	0.04

Cycle Length: 90

Actuated Cycle Length: 85.1

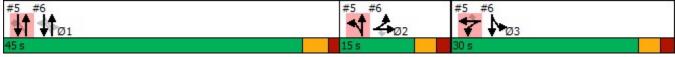
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.24

Intersection Signal Delay: 14.5 Intersection LOS: B
Intersection Capacity Utilization 71.8% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street





Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

Reference No.: 5999

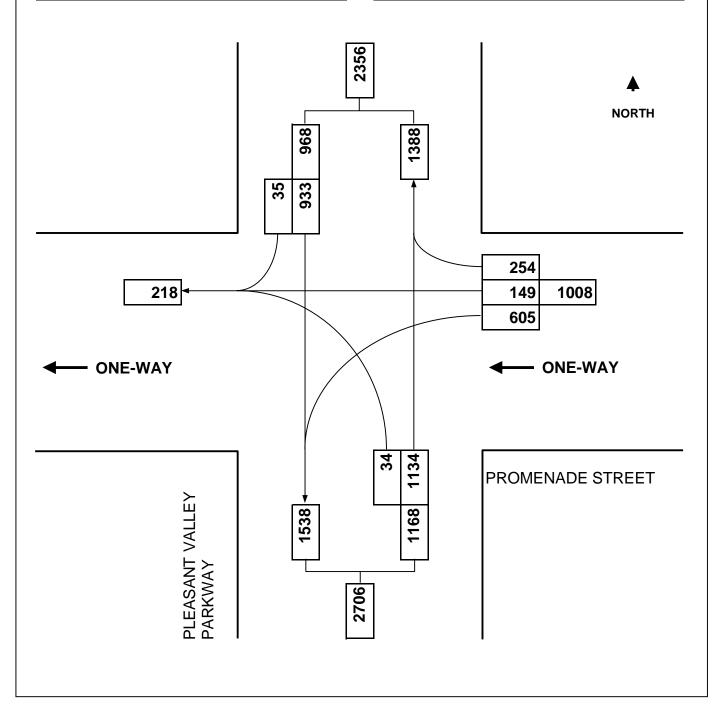
Existing: PM Peak Hour

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: 4:30 PM - 5:30 PM

Future: n/a



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				1	सी	7		414			^	7
Traffic Volume (vph)	0	0	0	605	149	254	34	1134	0	0	933	35
Future Volume (vph)	0	0	0	605	149	254	34	1134	0	0	933	35
Satd. Flow (prot)	0	0	0	1665	1715	1583	0	3606	0	0	3610	1615
Flt Permitted				0.950	0.971			0.911				
Satd. Flow (perm)	0	0	0	1665	1715	1583	0	3289	0	0	3610	1615
Satd. Flow (RTOR)						85						85
Lane Group Flow (vph)	0	0	0	387	391	262	0	1179	0	0	962	36
Turn Type				Split	NA	Perm	D.P+P	NA			NA	Perm
Protected Phases				3	3		2	12			1	
Permitted Phases						3	1					1
Total Split (s)				35.0	35.0	35.0	15.0				40.0	40.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				28.1	28.1	28.1		45.1			35.1	35.1
Actuated g/C Ratio				0.32	0.32	0.32		0.51			0.40	0.40
v/c Ratio				0.73	0.72	0.47		0.69			0.67	0.05
Control Delay				35.7	34.8	18.7		1.5			25.1	0.1
Queue Delay				0.0	0.0	0.0		1.8			0.0	0.0
Total Delay				35.7	34.8	18.7		3.3			25.1	0.1
LOS				D	С	В		Α			С	Α
Approach Delay					31.1			3.3			24.2	
Approach LOS					С			Α			С	
Queue Length 50th (ft)				198	200	75		5			234	0
Queue Length 95th (ft)				309	308	145		m8			305	1
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				567	584	595		1717			1435	693
Starvation Cap Reductn				0	0	0		354			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				0.68	0.67	0.44		0.87			0.67	0.05

Cycle Length: 90

Actuated Cycle Length: 88.2

Control Type: Actuated-Uncoordinated

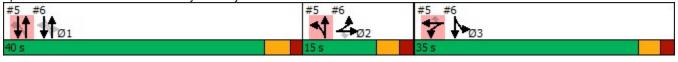
Maximum v/c Ratio: 1.28

Intersection Signal Delay: 18.8 Intersection LOS: B
Intersection Capacity Utilization 84.9% ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street



Kinsley Avenue at Acorn Street





Major Street: Kinsley Street

City/Town: Providence, RI

Reference No.: 5999

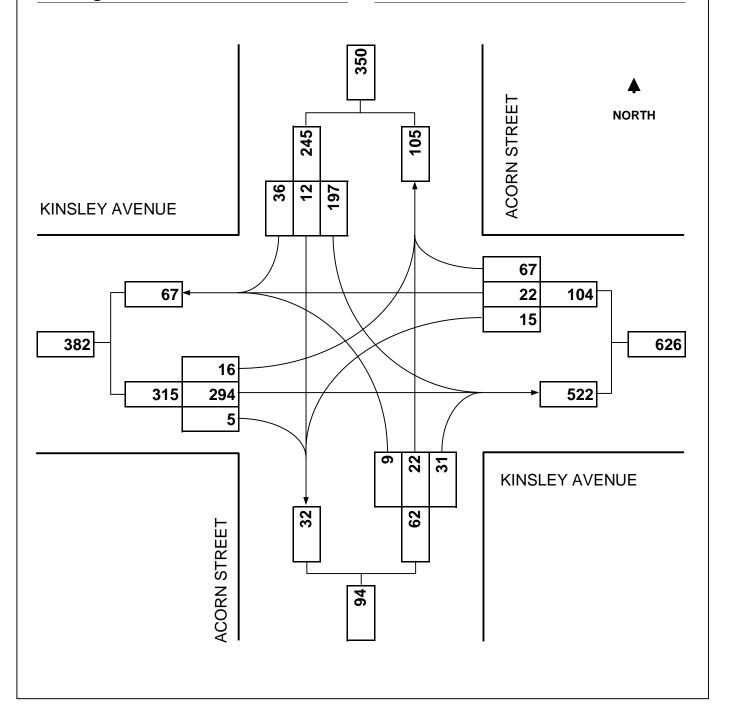
Existing: AM Peak Hour

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: 8:00 AM - 9:00 AM

Future: n/a



Intersection	
Intersection Delay, s/veh Intersection LOS	11.4
ntersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	16	294	5	15	22	67	9	22	31	197	12	36
Future Vol, veh/h	16	294	5	15	22	67	9	22	31	197	12	36
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	320	5	16	24	73	10	24	34	214	13	39
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	12.4			8.9			8.9			11.7		
HCM LOS	В			Α			Α			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	15%	5%	14%	80%
Vol Thru, %	35%	93%	21%	5%
Vol Right, %	50%	2%	64%	15%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	62	315	104	245
LT Vol	9	16	15	197
Through Vol	22	294	22	12
RT Vol	31	5	67	36
Lane Flow Rate	67	342	113	266
Geometry Grp	1	1	1	1
Degree of Util (X)	0.099	0.469	0.157	0.392
Departure Headway (Hd)	5.281	5.041	4.997	5.301
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	681	719	719	684
Service Time	3.297	3.041	3.016	3.301
HCM Lane V/C Ratio	0.098	0.476	0.157	0.389
HCM Control Delay	8.9	12.4	8.9	11.7
HCM Lane LOS	Α	В	Α	В
HCM 95th-tile Q	0.3	2.5	0.6	1.9

Existing Traffic Conditions
Timing Plan: AM Peak Hour



Major Street: Kinsley Street

City/Town: Providence, RI

Reference No.: 5999

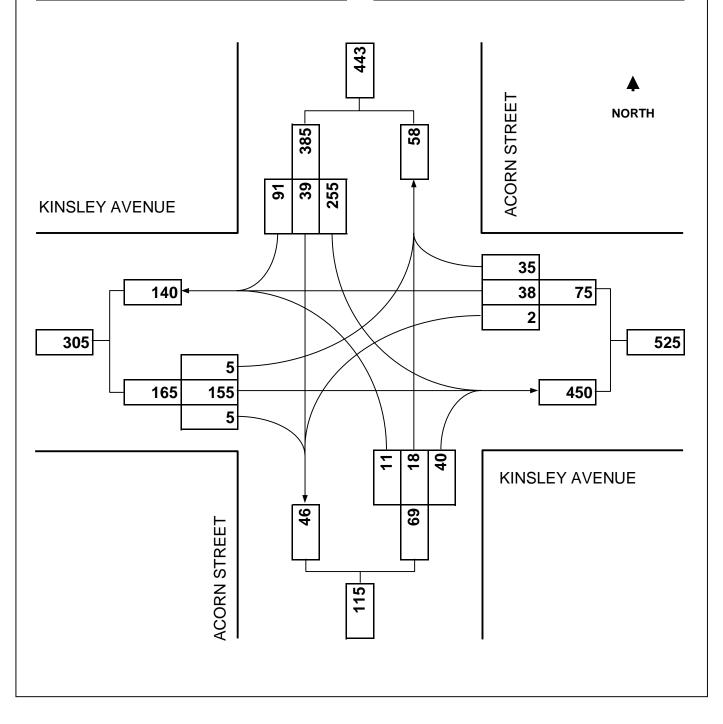
Existing: PM Peak Hour

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: 4:30 PM - 5:30 PM

Future: n/a



Intersection Delay, s/veh	11.4
ntersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	5	155	5	2	38	35	11	18	40	255	39	91
Future Vol, veh/h	5	155	5	2	38	35	11	18	40	255	39	91
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	2	0	0	2	0	0	0	0	0	0	0
Mvmt Flow	5	168	5	2	41	38	12	20	43	277	42	99
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	10			8.8			8.4			13.1		
HCM LOS	Α			Α			Α			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	16%	3%	3%	66%	
Vol Thru, %	26%	94%	51%	10%	
Vol Right, %	58%	3%	47%	24%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	69	165	75	385	
LT Vol	11	5	2	255	
Through Vol	18	155	38	39	
RT Vol	40	5	35	91	
Lane Flow Rate	75	179	82	418	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.099	0.257	0.114	0.539	
Departure Headway (Hd)	4.752	5.155	5.042	4.639	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	746	690	703	774	
Service Time	2.838	3.234	3.134	2.699	
HCM Lane V/C Ratio	0.101	0.259	0.117	0.54	
HCM Control Delay	8.4	10	8.8	13.1	
HCM Lane LOS	Α	Α	Α	В	
HCM 95th-tile Q	0.3	1	0.4	3.3	

Existing Traffic Conditions
Timing Plan: PM Peak Hour

Pro	nosed	Commer	rial Red	eveloi	nment
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Appendix

Providence, Rhode Island

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Future 2023 No-Build Weekday AM / PM Peak Hour

Dean Street at Kinsley Avenue/Providence Place
Pleasant Valley Parkway at Promenade Street
Kinsley Avenue at Acorn Street

Dean Street at Kinsley Avenue/Providence Place





Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

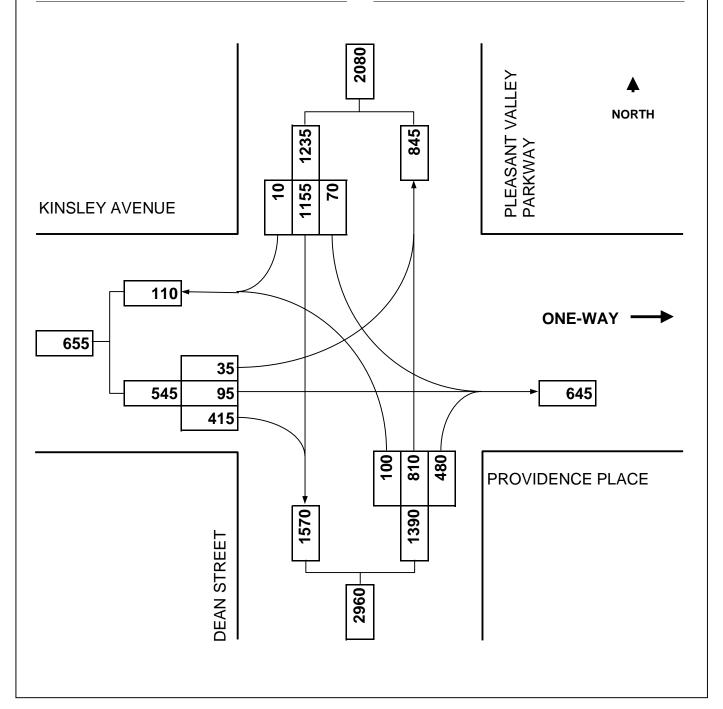
Existing: n/a

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 No Build



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ન	7				7	†		7	↑ ↑	
Traffic Volume (vph)	35	95	415	0	0	0	100	810	480	70	1155	10
Future Volume (vph)	35	95	415	0	0	0	100	810	480	70	1155	10
Satd. Flow (prot)	0	1875	1599	0	0	0	1805	3366	0	1805	3536	0
Flt Permitted		0.987					0.095			0.095		
Satd. Flow (perm)	0	1875	1599	0	0	0	180	3366	0	180	3536	0
Satd. Flow (RTOR)			113					181			2	
Lane Group Flow (vph)	0	135	432	0	0	0	103	1330	0	71	1189	0
Turn Type	Split	NA	Perm				Perm	NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2				1			1		
Total Split (s)	23.0	23.0	23.0				47.0	47.0		20.0		
Total Lost Time (s)		5.0	5.0				5.0	5.0		5.0		
Act Effct Green (s)		18.0	18.0				42.0	42.0		57.0	62.0	
Actuated g/C Ratio		0.20	0.20				0.47	0.47		0.63	0.69	
v/c Ratio		0.36	1.05				1.23	0.80		0.18	0.49	
Control Delay		34.3	87.3				199.7	21.8		9.6	1.5	
Queue Delay		0.0	0.0				0.0	0.0		0.9	0.3	
Total Delay		34.3	87.3				199.7	21.8		10.5	1.8	
LOS		С	F				F	С		В	Α	
Approach Delay		74.7						34.6			2.2	
Approach LOS		Е						С			Α	
Queue Length 50th (ft)		67	~215				~73	284		2	0	
Queue Length 95th (ft)		120	#400				#130	376		m12	0	
Internal Link Dist (ft)		488			721			853			84	
Turn Bay Length (ft)			100				175			100		
Base Capacity (vph)		375	410				84	1667		384	2436	
Starvation Cap Reductn		0	0				0	0		169	557	
Spillback Cap Reductn		0	0				0	0		0	0	
Storage Cap Reductn		0	0				0	0		0	0	
Reduced v/c Ratio		0.36	1.05				1.23	0.80		0.33	0.63	

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.23

Intersection Signal Delay: 29.1

Intersection LOS: C ICU Level of Service C

Intersection Capacity Utilization 66.3%

Analysis Period (min) 15 Volume exceeds capacity, queue is theoretically infinite.

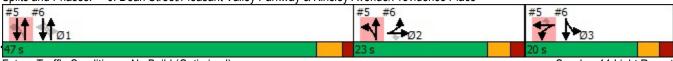
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Dean Street/Pleasant Valley Parkway & Kinsley Avenue/Providence Place



Future Traffic Conditions - No Build (Optimized)

Timing Plan: AM Peak Hour



Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

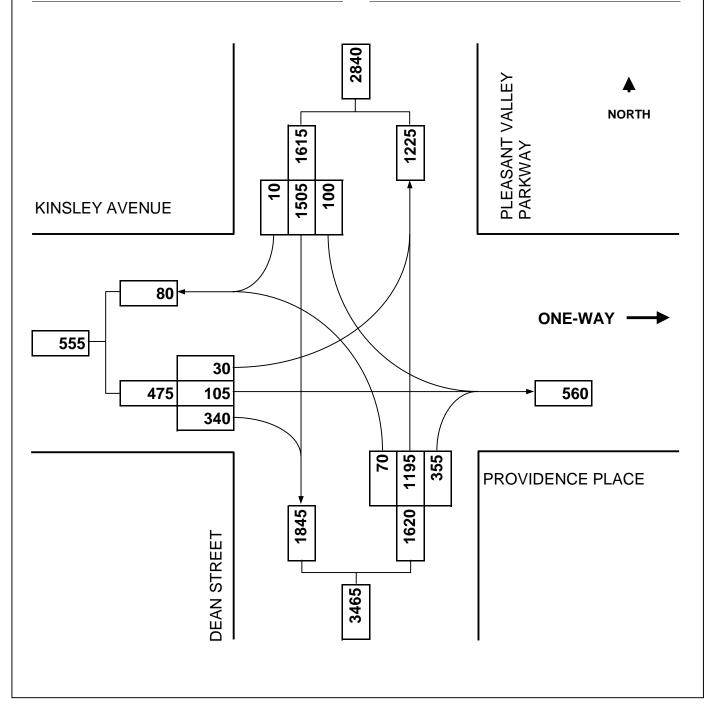
Existing: n/a

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: PM Peak Hour

Future: 2023 No Build



	•	-	•	•	•	•	1	†	-	-	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7				7	†		7	†	
Traffic Volume (vph)	30	105	340	0	0	0	70	1195	355	100	1505	10
Future Volume (vph)	30	105	340	0	0	0	70	1195	355	100	1505	10
Satd. Flow (prot)	0	1879	1599	0	0	0	1805	3434	0	1805	3536	0
Flt Permitted		0.989					0.100			0.100		
Satd. Flow (perm)	0	1879	1599	0	0	0	190	3434	0	190	3536	0
Satd. Flow (RTOR)			85					55			2	
Lane Group Flow (vph)	0	140	354	0	0	0	72	1598	0	102	1546	0
Turn Type	Split	NA	Perm				Perm	NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2				1			1		
Total Split (s)	19.0	19.0	19.0				45.0	45.0		26.0		
Total Lost Time (s)		5.0	5.0				5.0	5.0		5.0		
Act Effct Green (s)		14.0	14.0				40.0	40.0		61.0	66.0	
Actuated g/C Ratio		0.16	0.16				0.44	0.44		0.68	0.73	
v/c Ratio		0.48	1.11				0.86	1.03		0.20	0.60	
Control Delay		40.9	111.3				94.9	55.3		6.3	1.2	
Queue Delay		0.0	0.0				0.0	0.0		1.5	0.5	
Total Delay		40.9	111.3				94.9	55.3		7.8	1.6	
LOS		D	F				F	Е		Α	Α	
Approach Delay		91.4						57.0			2.0	
Approach LOS		F						Е			Α	
Queue Length 50th (ft)		74	~188				36	~506		3	0	
Queue Length 95th (ft)		132	#360				#122	#644		m8	m0	
Internal Link Dist (ft)		488			721			853			84	
Turn Bay Length (ft)			100				175			100		
Base Capacity (vph)		292	320				84	1556		505	2593	
Starvation Cap Reductn		0	0				0	0		270	525	
Spillback Cap Reductn		0	0				0	0		0	0	
Storage Cap Reductn		0	0				0	0		0	0	
Reduced v/c Ratio		0.48	1.11				0.86	1.03		0.43	0.75	

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.11

Intersection Signal Delay: 37.7

Intersection LOS: D
ICU Level of Service C

Intersection Capacity Utilization 71.3%

Analysis Period (min) 15

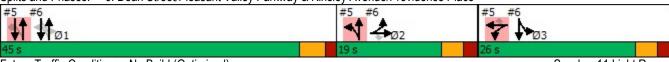
Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Dean Street/Pleasant Valley Parkway & Kinsley Avenue/Providence Place



Future Traffic Conditions - No Build (Optimized)

Timing Plan: PM Peak Hour

Pleasant Valley Parkway at Promenade Street





Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

Reference No.: 5999

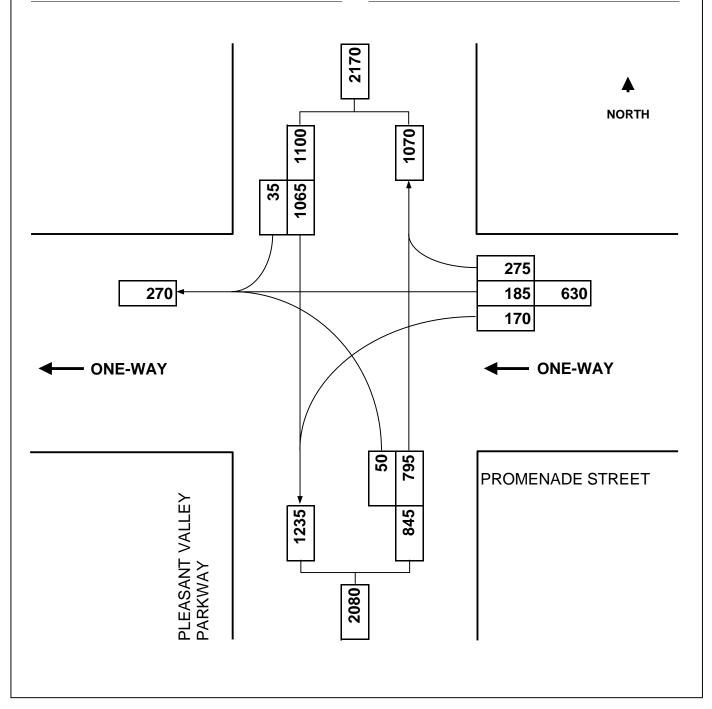
Existing: n/a

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 No Build



	۶	→	•	•	•	•	1	†	-	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				7	र्स	7		414			^	7
Traffic Volume (vph)	0	0	0	170	185	275	50	795	0	0	1065	35
Future Volume (vph)	0	0	0	170	185	275	50	795	0	0	1065	35
Satd. Flow (prot)	0	0	0	1698	1796	1615	0	3533	0	0	3574	1615
Flt Permitted				0.950	0.996			0.836				
Satd. Flow (perm)	0	0	0	1698	1796	1615	0	2962	0	0	3574	1615
Satd. Flow (RTOR)						228						85
Lane Group Flow (vph)	0	0	0	157	209	284	0	854	0	0	1098	36
Turn Type				Split	NA	Perm	D.P+P	NA			NA	Perm
Protected Phases				3	3		2	12			1	
Permitted Phases						3	1					1
Total Split (s)				20.0	20.0	20.0	23.0				47.0	47.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				15.0	15.0	15.0		60.0			42.0	42.0
Actuated g/C Ratio				0.17	0.17	0.17		0.67			0.47	0.47
v/c Ratio				0.55	0.70	0.62		0.41			0.66	0.05
Control Delay				42.8	49.3	15.0		1.0			20.9	0.1
Queue Delay				0.0	0.0	0.0		0.5			0.0	0.0
Total Delay				42.8	49.3	15.0		1.5			20.9	0.1
LOS				D	D	В		Α			С	Α
Approach Delay					32.7			1.5			20.2	
Approach LOS					С			Α			С	
Queue Length 50th (ft)				87	118	28		8			243	0
Queue Length 95th (ft)				153	#217	107		8			313	1
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				283	299	459		2088			1667	799
Starvation Cap Reductn				0	0	0		740			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				0.55	0.70	0.62		0.63			0.66	0.05

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.23

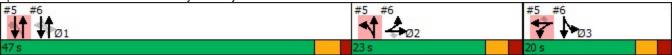
Intersection Signal Delay: 17.3 Intersection LOS: B
Intersection Capacity Utilization 75.1% ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street





Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

Reference No.: 5999

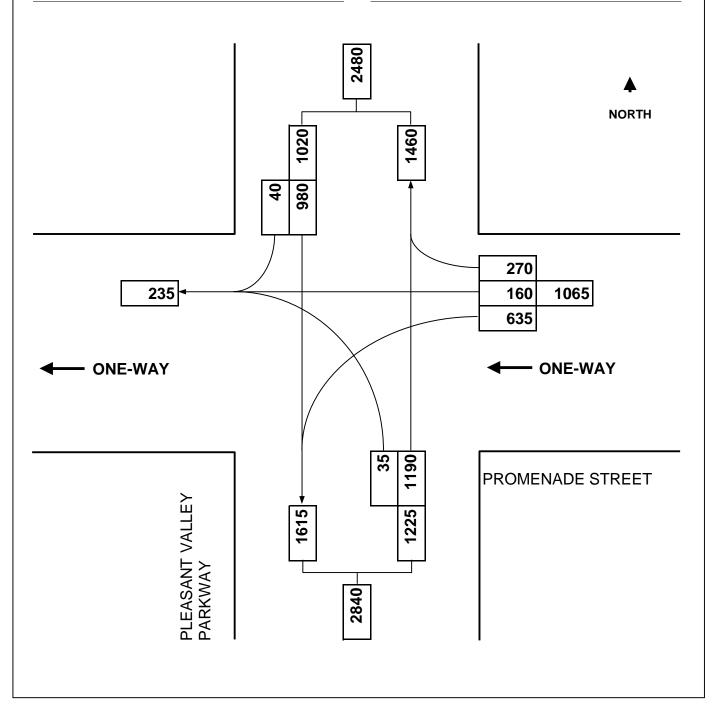
Existing: n/a

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: PM Peak Hour

Future: 2023 No Build



	۶	→	•	•	•	•	1	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				1	ર્ન	7		414			^	7
Traffic Volume (vph)	0	0	0	635	160	270	35	1190	0	0	980	40
Future Volume (vph)	0	0	0	635	160	270	35	1190	0	0	980	40
Satd. Flow (prot)	0	0	0	1665	1715	1583	0	3606	0	0	3610	1615
Flt Permitted				0.950	0.971			0.912				
Satd. Flow (perm)	0	0	0	1665	1715	1583	0	3292	0	0	3610	1615
Satd. Flow (RTOR)						85						85
Lane Group Flow (vph)	0	0	0	406	414	278	0	1237	0	0	1010	41
Turn Type				Split	NA	Perm	D.P+P	NA			NA	Perm
Protected Phases				3	3		2	12			1	
Permitted Phases						3	1					1
Total Split (s)				26.0	26.0	26.0	19.0				45.0	45.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				21.0	21.0	21.0		54.0			40.0	40.0
Actuated g/C Ratio				0.23	0.23	0.23		0.60			0.44	0.44
v/c Ratio				1.05	1.03	0.64		0.61			0.63	0.05
Control Delay				94.1	90.3	29.0		0.7			21.5	0.7
Queue Delay				0.0	0.0	0.0		1.3			0.0	0.0
Total Delay				94.1	90.3	29.0		2.0			21.5	0.7
LOS				F	F	С		Α			С	Α
Approach Delay					76.2			2.0			20.7	
Approach LOS					Е			Α			С	
Queue Length 50th (ft)				~266	~269	98		4			225	0
Queue Length 95th (ft)				#452	#456	184		m4			291	4
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				388	400	434		2024			1604	765
Starvation Cap Reductn				0	0	0		537			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				1.05	1.03	0.64		0.83			0.63	0.05

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.11

Intersection Signal Delay: 31.8
Intersection Capacity Utilization 88.3%

Intersection LOS: C
ICU Level of Service E

Analysis Period (min) 15

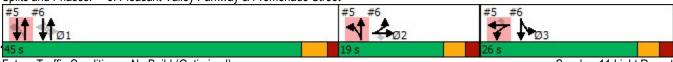
Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street



Future Traffic Conditions - No Build (Optimized)

Timing Plan: PM Peak Hour

Kinsley Avenue at Acorn Street





Major Street: Kinsley Street

City/Town: Providence, RI

Reference No.: 5999

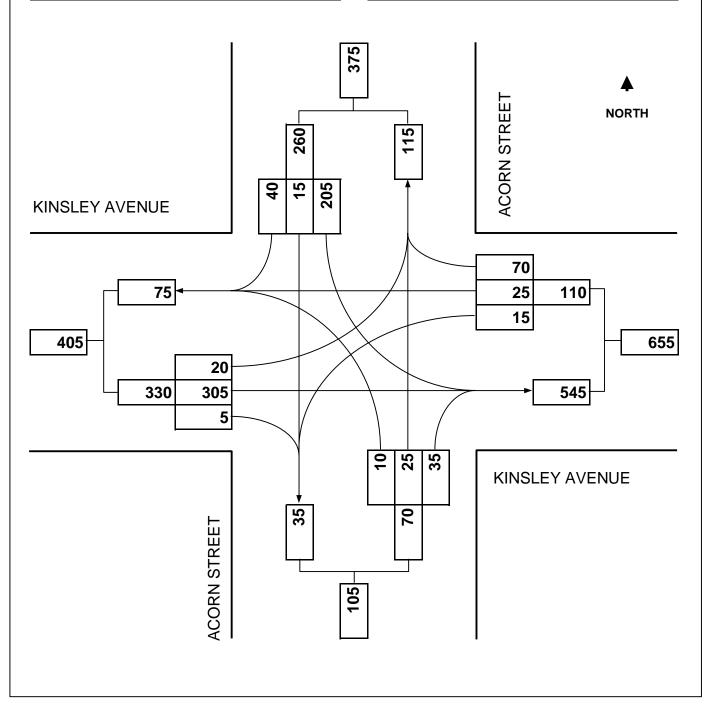
Existing: n/a

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 No Build



ersection	
	12
ersection Delay, s/veh	12
ersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	20	305	5	15	25	70	10	25	35	205	15	40
Future Vol, veh/h	20	305	5	15	25	70	10	25	35	205	15	40
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	332	5	16	27	76	11	27	38	223	16	43
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	13.4			9.2			9.2			12.3		
HCM LOS	В			Α			Α			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	14%	6%	14%	79%	
Vol Thru, %	36%	92%	23%	6%	
Vol Right, %	50%	2%	64%	15%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	70	330	110	260	
LT Vol	10	20	15	205	
Through Vol	25	305	25	15	
RT Vol	35	5	70	40	
Lane Flow Rate	76	359	120	283	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.114	0.513	0.17	0.422	
Departure Headway (Hd)	5.405	5.144	5.125	5.382	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	662	707	699	670	
Service Time	3.45	3.144	3.163	3.418	
HCM Lane V/C Ratio	0.115	0.508	0.172	0.422	
HCM Control Delay	9.2	13.4	9.2	12.3	
HCM Lane LOS	Α	В	Α	В	
HCM 95th-tile Q	0.4	3	0.6	2.1	



Major Street: Kinsley Street

City/Town: Providence, RI

Reference No.: 5999

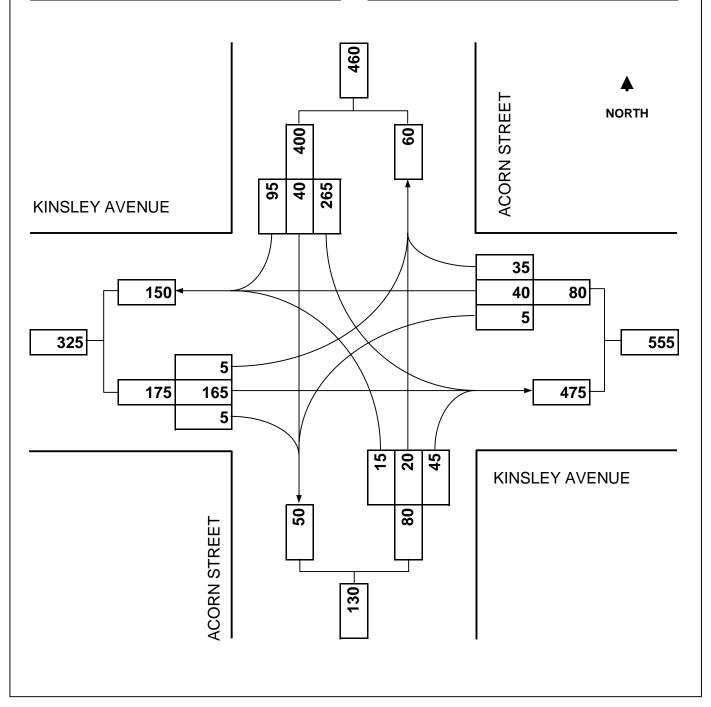
Existing: n/a

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: PM Peak Hour

Future: 2023 No Build



ntersection	
ntersection Delay, s/veh	12
ntersection Delay, s/veh ntersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	5	165	5	5	40	35	15	20	45	265	40	95
Future Vol, veh/h	5	165	5	5	40	35	15	20	45	265	40	95
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	2	0	0	2	0	0	0	0	0	0	0
Mvmt Flow	5	179	5	5	43	38	16	22	49	288	43	103
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	10.4			9			8.6			13.9		
HCM LOS	В			Α			Α			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	19%	3%	6%	66%
Vol Thru, %	25%	94%	50%	10%
Vol Right, %	56%	3%	44%	24%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	80	175	80	400
LT Vol	15	5	5	265
Through Vol	20	165	40	40
RT Vol	45	5	35	95
Lane Flow Rate	87	190	87	435
Geometry Grp	1	1	1	1
Degree of Util (X)	0.12	0.277	0.127	0.568
Departure Headway (Hd)	4.952	5.242	5.274	4.706
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	728	677	683	761
Service Time	2.956	3.342	3.28	2.783
HCM Lane V/C Ratio	0.12	0.281	0.127	0.572
HCM Control Delay	8.6	10.4	9	13.9
HCM Lane LOS	Α	В	Α	В
HCM 95th-tile Q	0.4	1.1	0.4	3.6

D

Future 2023 Build Weekday AM / PM Peak Hour

Dean Street at Kinsley Avenue/Providence Place
Pleasant Valley Parkway at Promenade Street
Kinsley Avenue at Acorn Street
Kinsley Avenue at Western Site Driveway
Kinsley Avenue at Eastern Site Driveway



Dean Street at Kinsley Avenue/Providence Place





Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

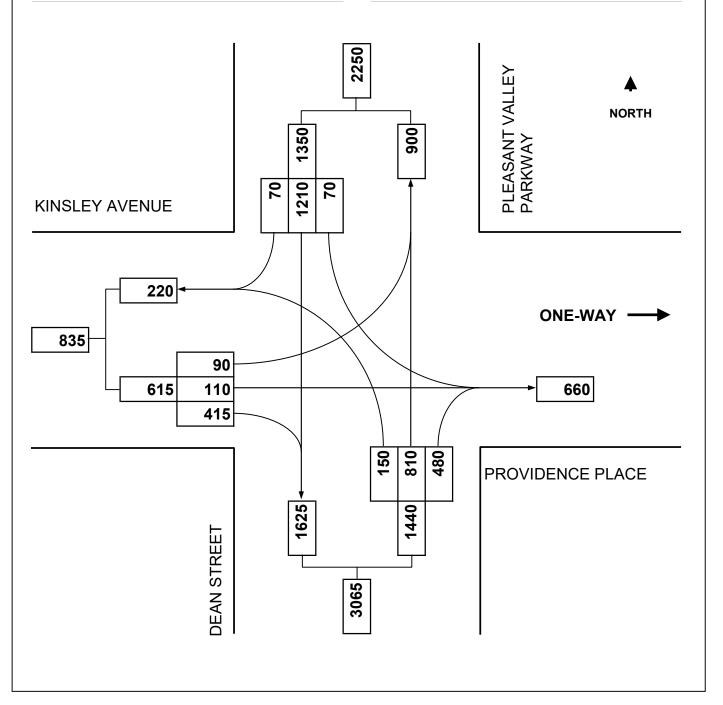
Existing: n/a

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 Build



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7				7	†		7	†	
Traffic Volume (vph)	90	110	415	0	0	0	150	810	480	70	1210	70
Future Volume (vph)	90	110	415	0	0	0	150	810	480	70	1210	70
Satd. Flow (prot)	0	1858	1599	0	0	0	1805	3366	0	1805	3515	0
Flt Permitted		0.978					0.095			0.095		
Satd. Flow (perm)	0	1858	1599	0	0	0	180	3366	0	180	3515	0
Satd. Flow (RTOR)			101					181			15	
Lane Group Flow (vph)	0	209	432	0	0	0	155	1330	0	71	1306	0
Turn Type	Split	NA	Perm				Perm	NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2				1			1		
Total Split (s)	23.0	23.0	23.0				47.0	47.0		20.0		
Total Lost Time (s)		5.0	5.0				5.0	5.0		5.0		
Act Effct Green (s)		18.0	18.0				42.0	42.0		57.0	62.0	
Actuated g/C Ratio		0.20	0.20				0.47	0.47		0.63	0.69	
v/c Ratio		0.56	1.08				1.85	0.80		0.18	0.54	
Control Delay		39.2	96.7				445.0	21.8		8.7	1.5	
Queue Delay		0.0	0.0				0.0	0.0		0.9	0.4	
Total Delay		39.2	96.7				445.0	21.8		9.6	1.9	
LOS		D	F				F	С		Α	Α	
Approach Delay		77.9						66.0			2.3	
Approach LOS		Е						Е			Α	
Queue Length 50th (ft)		108	~227				~135	284		2	0	
Queue Length 95th (ft)		179	#412				#214	376		m6	0	
Internal Link Dist (ft)		115			721			853			84	
Turn Bay Length (ft)			50				175			100		
Base Capacity (vph)		371	400				84	1667		384	2426	
Starvation Cap Reductn		0	0				0	0		170	528	
Spillback Cap Reductn		0	0				0	0		0	0	
Storage Cap Reductn		0	0				0	0		0	0	
Reduced v/c Ratio		0.56	1.08				1.85	0.80		0.33	0.69	

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.85

Intersection Signal Delay: 43.1

Intersection LOS: D ICU Level of Service C

Intersection Capacity Utilization 69.7%

Analysis Period (min) 15

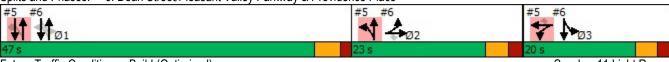
 Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Dean Street/Pleasant Valley Parkway & Providence Place



Future Traffic Conditions - Build (Optimized)

Timing Plan: AM Peak Hour



Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

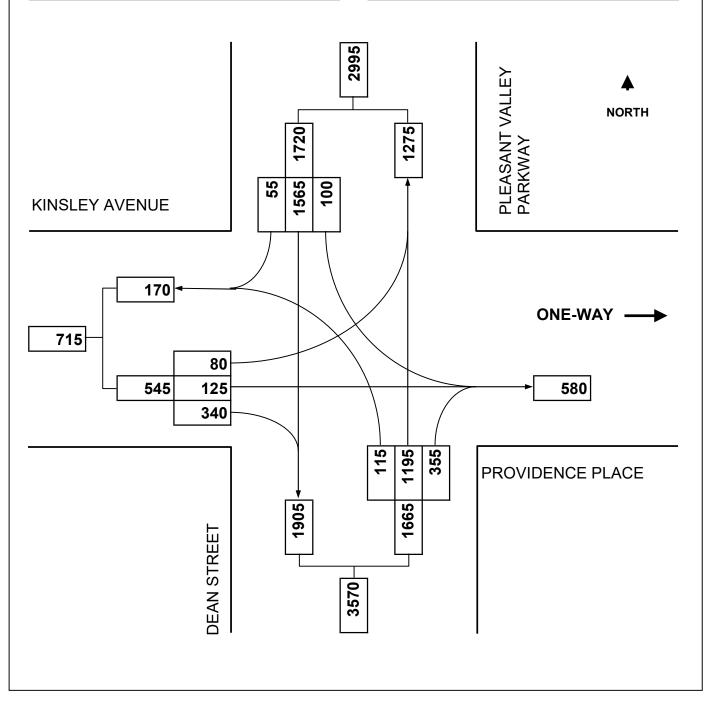
Existing: n/a

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: PM Peak Hour

Future: 2023 Build



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7				7	†		*	†	
Traffic Volume (vph)	80	125	340	0	0	0	115	1195	355	100	1565	55
Future Volume (vph)	80	125	340	0	0	0	115	1195	355	100	1565	55
Satd. Flow (prot)	0	1864	1599	0	0	0	1805	3434	0	1805	3524	0
Flt Permitted		0.981					0.105			0.105		
Satd. Flow (perm)	0	1864	1599	0	0	0	200	3434	0	200	3524	0
Satd. Flow (RTOR)			85					53			8	
Lane Group Flow (vph)	0	213	354	0	0	0	119	1598	0	102	1653	0
Turn Type	Split	NA	Perm				Perm	NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2				1			1		
Total Split (s)	26.0	26.0	26.0				43.0	43.0		21.0		
Total Lost Time (s)		5.0	5.0				5.0	5.0		5.0		
Act Effct Green (s)		19.8	19.8				38.0	38.0		54.0	59.0	
Actuated g/C Ratio		0.22	0.22				0.43	0.43		0.61	0.66	
v/c Ratio		0.51	0.84				1.40	1.07		0.25	0.71	
Control Delay		35.2	43.9				262.6	68.7		7.5	1.5	
Queue Delay		0.0	0.0				0.0	0.0		1.4	1.3	
Total Delay		35.2	43.9				262.6	68.7		8.9	2.8	
LOS		D	D				F	Е		Α	Α	
Approach Delay		40.6						82.2			3.1	
Approach LOS		D						F			Α	
Queue Length 50th (ft)		105	147				~92	~531		2	1	
Queue Length 95th (ft)		175	#291				#155	#669		m2	m0	
Internal Link Dist (ft)		115			721			853			84	
Turn Bay Length (ft)			50				175			100		
Base Capacity (vph)		440	443				85	1500		411	2344	
Starvation Cap Reductn		0	0				0	0		181	443	
Spillback Cap Reductn		0	0				0	0		0	0	
Storage Cap Reductn		0	0				0	0		0	0	
Reduced v/c Ratio		0.48	0.80				1.40	1.07		0.44	0.87	

Cycle Length: 90

Actuated Cycle Length: 88.8

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.40

Intersection Signal Delay: 42.0 Intersection Capacity Utilization 74.9%

Intersection LOS: D ICU Level of Service D

Analysis Period (min) 15

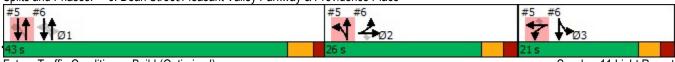
 Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Dean Street/Pleasant Valley Parkway & Providence Place



Future Traffic Conditions - Build (Optimized)

Timing Plan: PM Peak Hour

Pleasant Valley Parkway at Promenade Street





Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

Reference No.: 5999

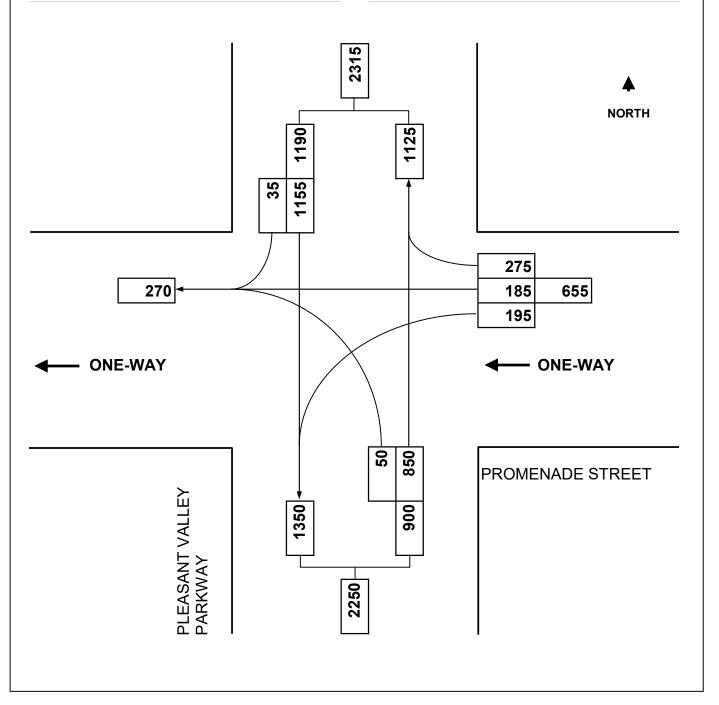
Existing: n/a

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 Build



	•	→	•	•	•	•	1	1	1	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				7	र्स	7		414			^	7
Traffic Volume (vph)	0	0	0	195	185	275	50	850	0	0	1155	35
Future Volume (vph)	0	0	0	195	185	275	50	850	0	0	1155	35
Satd. Flow (prot)	0	0	0	1665	1775	1583	0	3599	0	0	3610	1615
Flt Permitted				0.950	0.995			0.808				
Satd. Flow (perm)	0	0	0	1665	1775	1583	0	2917	0	0	3610	1615
Satd. Flow (RTOR)						205						85
Lane Group Flow (vph)	0	0	0	181	211	284	0	910	0	0	1191	36
Turn Type				Split	NA	Perm	D.P+P	NA			NA	Perm
Protected Phases				3	3		2	12			1	
Permitted Phases						3	1					1
Total Split (s)				20.0	20.0	20.0	23.0				47.0	47.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				15.0	15.0	15.0		60.0			42.0	42.0
Actuated g/C Ratio				0.17	0.17	0.17		0.67			0.47	0.47
v/c Ratio				0.65	0.72	0.65		0.44			0.71	0.05
Control Delay				47.5	50.5	18.8		1.1			22.0	0.1
Queue Delay				0.0	0.0	0.0		0.5			0.0	0.0
Total Delay				47.5	50.5	18.8		1.6			22.0	0.1
LOS				D	D	В		Α			С	Α
Approach Delay					36.4			1.6			21.4	
Approach LOS					D			Α			С	
Queue Length 50th (ft)				102	121	40		9			273	0
Queue Length 95th (ft)				#189	#224	123		11			348	1
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				277	295	434		2081			1684	799
Starvation Cap Reductn				0	0	0		678			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				0.65	0.72	0.65		0.65			0.71	0.05

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.85

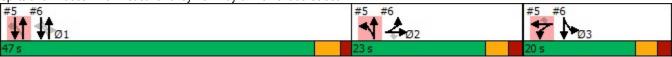
Intersection Signal Delay: 18.6 Intersection LOS: B
Intersection Capacity Utilization 79.5% ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street





Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

Reference No.: 5999

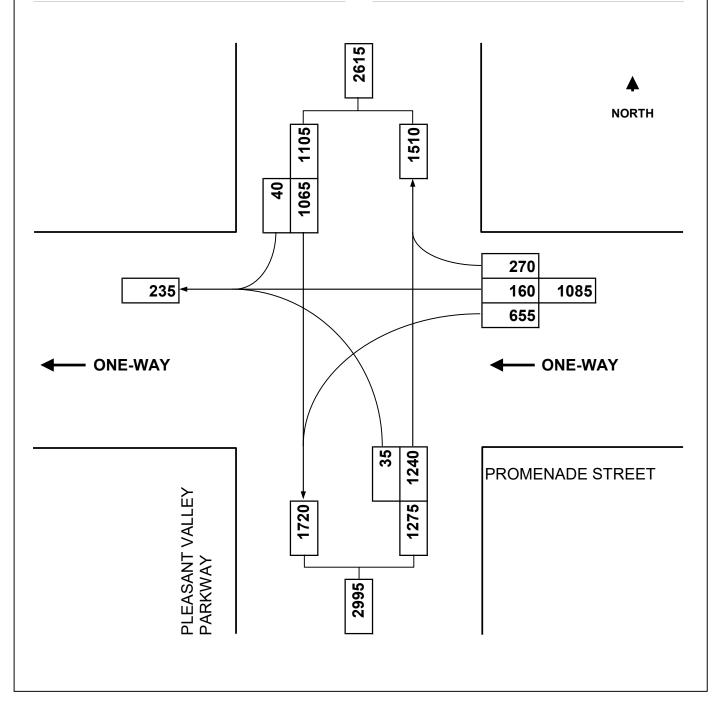
Existing: n/a

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: PM Peak Hour

Future: 2023 Build



	۶	→	•	•	←	•	1	†	-	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				1	ર્ન	7		414			^	7
Traffic Volume (vph)	0	0	0	655	160	270	35	1240	0	0	1065	40
Future Volume (vph)	0	0	0	655	160	270	35	1240	0	0	1065	40
Satd. Flow (prot)	0	0	0	1665	1713	1583	0	3606	0	0	3610	1615
Flt Permitted				0.950	0.970			0.917				
Satd. Flow (perm)	0	0	0	1665	1713	1583	0	3310	0	0	3610	1615
Satd. Flow (RTOR)						86						85
Lane Group Flow (vph)	0	0	0	418	422	278	0	1288	0	0	1098	41
Turn Type				Split	NA	Perm	D.P+P	NA			NA	Perm
Protected Phases				3	3		2	12			1	
Permitted Phases						3	1					1
Total Split (s)				21.0	21.0	21.0	26.0				43.0	43.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				16.0	16.0	16.0		57.8			38.0	38.0
Actuated g/C Ratio				0.18	0.18	0.18		0.65			0.43	0.43
v/c Ratio				1.39	1.37	0.78		0.58			0.71	0.06
Control Delay				227.5	217.2	41.1		0.9			24.3	0.7
Queue Delay				0.0	0.0	0.0		1.4			0.0	0.0
Total Delay				227.5	217.2	41.1		2.3			24.3	0.7
LOS				F	F	D		Α			С	Α
Approach Delay					177.3			2.3			23.4	
Approach LOS					F			Α			С	
Queue Length 50th (ft)				~340	~338	106		4			265	0
Queue Length 95th (ft)				#528	#528	#232		m4			341	4
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				300	308	356		2270			1545	740
Starvation Cap Reductn				0	0	0		731			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				1.39	1.37	0.78		0.84			0.71	0.06

Cycle Length: 90

Actuated Cycle Length: 88.8

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.40

Intersection Signal Delay: 64.3 Intersection Capacity Utilization 90.2% Intersection LOS: E
ICU Level of Service E

Analysis Period (min) 15

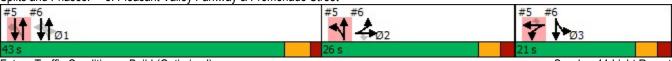
Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street



Future Traffic Conditions - Build (Optimized)

Timing Plan: PM Peak Hour

Kinsley Avenue at Acorn Street





Major Street: Kinsley Street

City/Town: Providence, RI

Reference No.: 5999

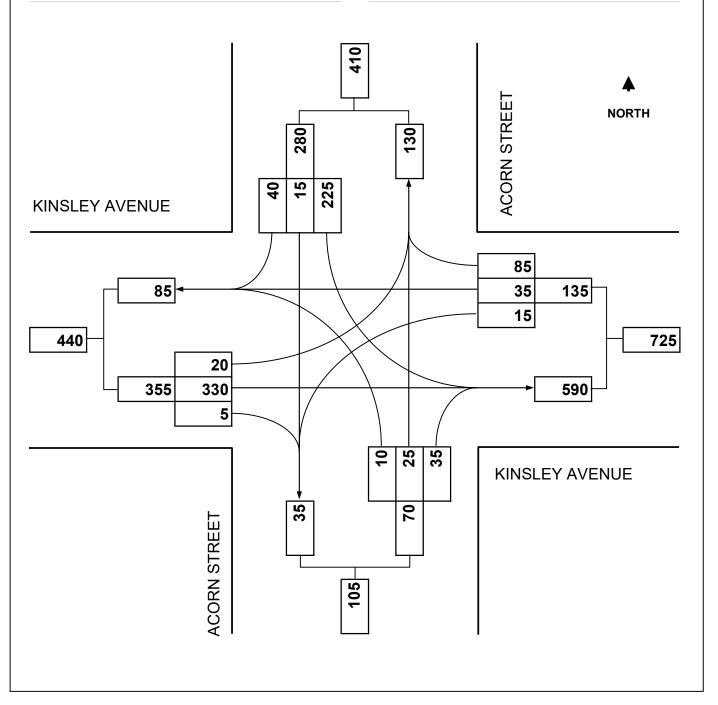
Existing: n/a

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 Build



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	20	330	5	15	35	85	10	25	35	225	15	40
Future Vol, veh/h	20	330	5	15	35	85	10	25	35	225	15	40
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	359	5	16	38	92	11	27	38	245	16	43
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	14.9			9.8			9.5			13.5		
HCM LOS	В			Α			Α			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	14%	6%	11%	80%	
Vol Thru, %	36%	93%	26%	5%	
Vol Right, %	50%	1%	63%	14%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	70	355	135	280	
LT Vol	10	20	15	225	
Through Vol	25	330	35	15	
RT Vol	35	5	85	40	
Lane Flow Rate	76	386	147	304	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.119	0.564	0.216	0.471	
Departure Headway (Hd)	5.653	5.265	5.289	5.569	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	632	683	676	645	
Service Time	3.71	3.302	3.336	3.609	
HCM Lane V/C Ratio	0.12	0.565	0.217	0.471	
HCM Control Delay	9.5	14.9	9.8	13.5	
HCM Lane LOS	А	В	Α	В	
HCM 95th-tile Q	0.4	3.5	0.8	2.5	



Major Street: Kinsley Street

City/Town: Providence, RI

Reference No.: 5999

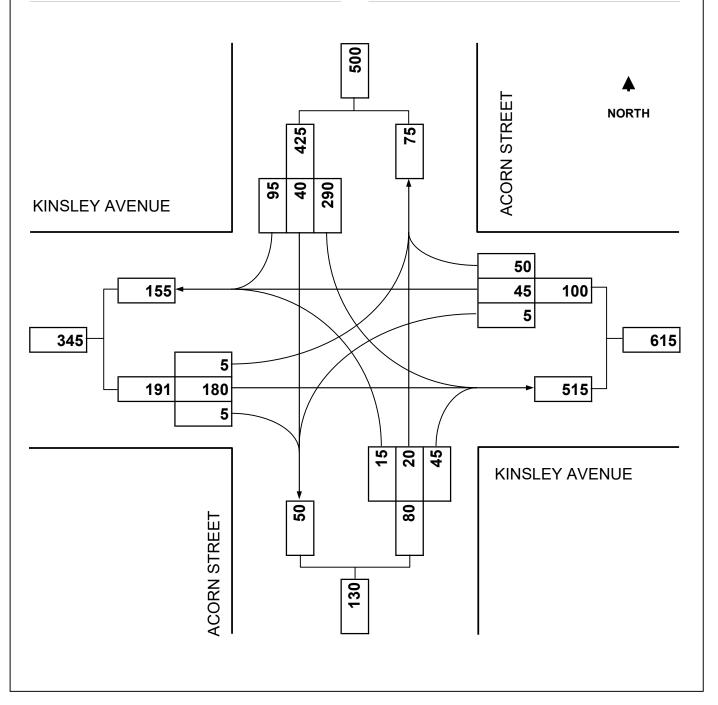
Existing: n/a

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: PM Peak Hour

Future: 2023 Build



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	5	185	5	5	45	50	15	20	45	290	40	95
Future Vol, veh/h	5	185	5	5	45	50	15	20	45	290	40	95
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	2	0	0	2	0	0	0	0	0	0	0
Mvmt Flow	5	201	5	5	49	54	16	22	49	315	43	103
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	11.1			9.5			9			16.2		
HCM LOS	В			Α			Α			С		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	19%	3%	5%	68%	
Vol Thru, %	25%	95%	45%	9%	
Vol Right, %	56%	3%	50%	22%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	80	195	100	425	
LT Vol	15	5	5	290	
Through Vol	20	185	45	40	
RT Vol	45	5	50	95	
Lane Flow Rate	87	212	109	462	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.125	0.323	0.163	0.636	
Departure Headway (Hd)	5.167	5.489	5.398	4.955	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	692	655	663	735	
Service Time	3.208	3.529	3.443	2.955	
HCM Lane V/C Ratio	0.126	0.324	0.164	0.629	
HCM Control Delay	9	11.1	9.5	16.2	
HCM Lane LOS	Α	В	Α	С	
HCM 95th-tile Q	0.4	1.4	0.6	4.6	

Kinsley Avenue at Western Site Driveway





Major Street: Kinsley Avenue

City/Town: Providence, RI

Reference No.: 5999

Existing: n/a

Minor Street: Western Site Driveway

Day of Week: Weekday

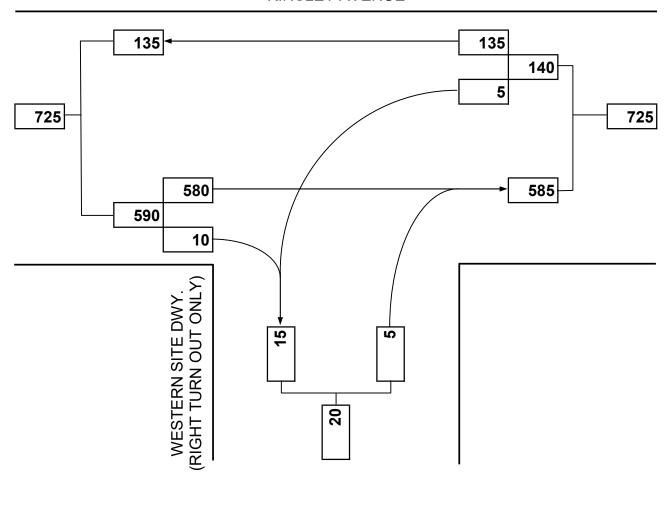
Peak Period: AM Peak Hour

Future: 2023 Build



NORTH

KINSLEY AVENUE



Intersection	0.1					
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽			र्स		7
Traffic Vol, veh/h	580	10	5	135	0	5
	580	10	5	135	0	5
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	† 0	-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	2	0	0
Mymt Flow	630	11	5	147	0	5
WWIIICTIOW	000		U	177	U	U
Major/Minor Ma	ajor1	N	/lajor2	- 1	Minor1	
Conflicting Flow All	0	0	641	0	-	636
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	4.1	-	-	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	_	-	_	-	-	-
Follow-up Hdwy	-	_	2.2	_	-	3.3
Pot Cap-1 Maneuver	_	-	953	_	0	481
Stage 1	_	_	-	_	0	-
Stage 2	_	_	_	_	0	_
Platoon blocked, %	_	_		_	•	
Mov Cap-1 Maneuver			953	_	_	481
Mov Cap-1 Maneuver	_	_	300	_	_	- U1
·	-	-	-	-	-	-
Stage 1		-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		12.6	
HCM LOS					В	
1.6		IDI 4	EDT		14/51	MACE
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		481	-	-	953	-
HCM Lane V/C Ratio		0.011	-	-	0.006	-
HCM Control Delay (s)		12.6	_	-	8.8	0
HCM Lane LOS HCM 95th %tile Q(veh)		B 0	-	-	A 0	Α



Major Street: Kinsley Avenue

City/Town: Providence, RI

Reference No.: 5999

Reference No.: 5999
Existing: n/a

Minor Street: Western Site Driveway

Day of Week: Weekday

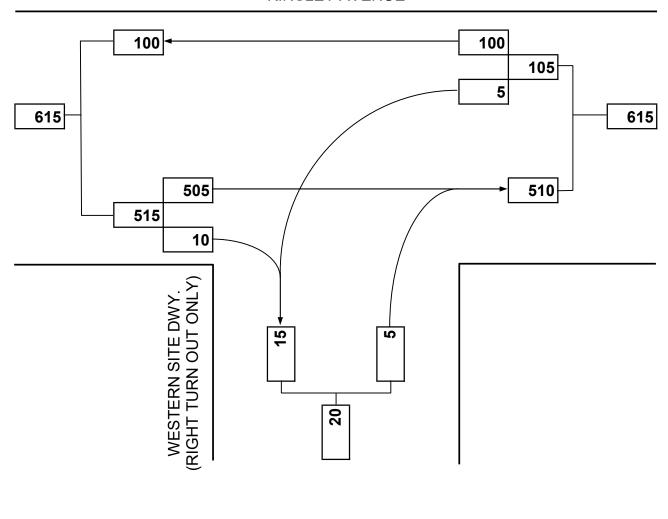
Peak Period: PM Peak Hour

Future: 2023 Build



NORTH

KINSLEY AVENUE



Intersection						
Int Delay, s/veh	0.2					
	EBT	EDD	\\/DI	\\/DT	NDI	NIDD
Movement		EBR	WBL	WBT	NBL	NBR
Lane Configurations	F 05	40	F	100	0	7
Traffic Vol, veh/h	505	10	5	100	0	5
Future Vol, veh/h	505	10	5	100	0	5
Conflicting Peds, #/hr	_ 0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	549	11	5	109	0	5
Major/Minor NA	laia-1		/nic=0		line=1	
	lajor1		Major2		/linor1	
Conflicting Flow All	0	0	560	0	-	555
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	4.1	-	-	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	2.2	-	-	3.3
Pot Cap-1 Maneuver	-	-	1021	-	0	535
Stage 1	-	-	-	-	0	-
Stage 2	-	-	-	-	0	-
Platoon blocked, %	-	_		_		
Mov Cap-1 Maneuver	-	-	1021	-	-	535
Mov Cap-2 Maneuver	-	_	_	_	-	-
Stage 1	_	_	_	_	_	_
Stage 2	_	_	_	_	_	_
Olage 2		_			-	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		11.8	
HCM LOS					В	
Minor Long/Major M.		JDI 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	- [NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		535	-		1021	-
HCM Lane V/C Ratio		0.01	-	-	0.005	-
HCM Control Delay (s)		11.8	-	-	8.5	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0	-	-	0	-

Kinsley Avenue at Eastern Site Driveway





Major Street: Kinsley Avenue

City/Town: Providence, RI

Reference No.: 5999

Existing: n/a

Minor Street: Eastern Site Driveway

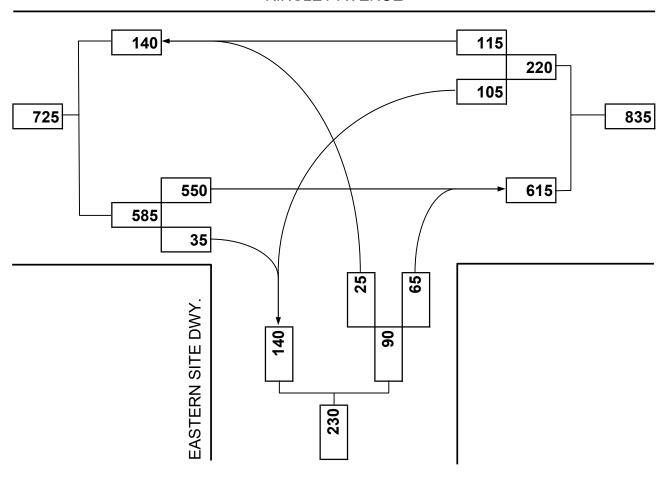
Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 Build

NORTH

KINSLEY AVENUE



Intersection						
Int Delay, s/veh	2.9					
		ED.	14/51	VA/D.T.	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1		40-	†	¥	
Traffic Vol, veh/h	550	35	105	115	25	65
Future Vol, veh/h	550	35	105	115	25	65
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	598	38	114	125	27	71
	lajor1		Major2		Minor1	
Conflicting Flow All	0	0	636	0	970	617
Stage 1	-	-	-	-	617	-
Stage 2	-	-	-	-	353	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	947	-	281	490
Stage 1	-	-	-	-	538	-
Stage 2	-	-	-	-	711	-
Platoon blocked, %	-	_		_		
Mov Cap-1 Maneuver	_	_	947	_	245	490
Mov Cap-2 Maneuver	_	_		_	245	-
Stage 1	-				538	_
Stage 2	_	-	-	-	619	-
Slaye 2	-	-	-	-	019	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		4.4		17.6	
HCM LOS					С	
		IDI (14/5-
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		383	-	-	947	-
HCM Lane V/C Ratio		0.255	-	-	0.121	-
HCM Control Delay (s)		17.6	-	-	9.3	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)		1	-	-	0.4	-



Major Street: Kinsley Avenue

City/Town: Providence, RI

Reference No.: 5999

Existing: n/a

Minor Street: Eastern Site Driveway

Day of Week: Weekday

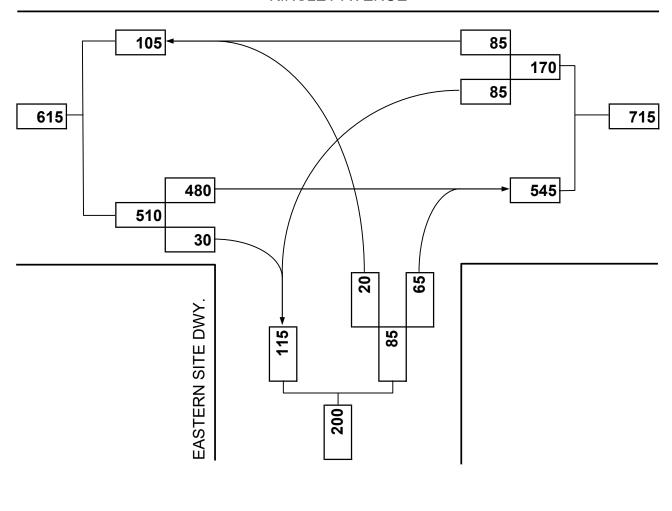
Peak Period: PM Peak Hour

Future: 2023 Build



NORTH

KINSLEY AVENUE



Intersection						
Int Delay, s/veh	2.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			<u></u>	¥	TIDIT
Traffic Vol, veh/h	480	30	85	85	20	65
Future Vol, veh/h	480	30	85	85	20	65
•	400	0	00	00	0	00
Conflicting Peds, #/hr						
•	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	522	33	92	92	22	71
NA - 1 - / NA1 NA			4 . 0		M'	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	555	0	815	539
Stage 1	-	-	-	-	539	-
Stage 2	-	-	-	-	276	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	_	_	5.42	-
Follow-up Hdwy	_	-	2.218	_	3.518	3.318
Pot Cap-1 Maneuver	_	_	1015	_	347	542
Stage 1	_	_	-	_	585	-
Stage 2	_	_	_	_	771	_
Platoon blocked, %	_	_	_		771	_
	-	_	101E	-	244	E40
Mov Cap-1 Maneuver	-	-	1015	-	314	542
Mov Cap-2 Maneuver	-	-	-	-	314	-
Stage 1	-	-	-	-	585	-
Stage 2	-	-	-	-	697	-
Approach	EB		WB		NB	
	0		4.5		14.7	
HCM LOS	U		4.3			
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		463	-	-	1015	
HCM Lane V/C Ratio		0.2	_		0.091	<u>-</u>
HCM Control Delay (s)		14.7			8.9	_
HCM Lane LOS		14.7 B		_	0.9 A	
			-			-
HCM 95th %tile Q(veh)		0.7	-	-	0.3	-

D

Future 2023 Build Alternative Weekday AM / PM Peak Hour

Pleasant Valley Parkway at Promenade Street

Kinsley Avenue at Acorn Street

Dean Street at Site Driveway

Kinsley Avenue at Western Site Driveway

Kinsley Avenue at Eastern Site Driveway



Dean Street at Kinsley Avenue/Providence Place





Major Street: Dean Street

City/Town: Providence, RI

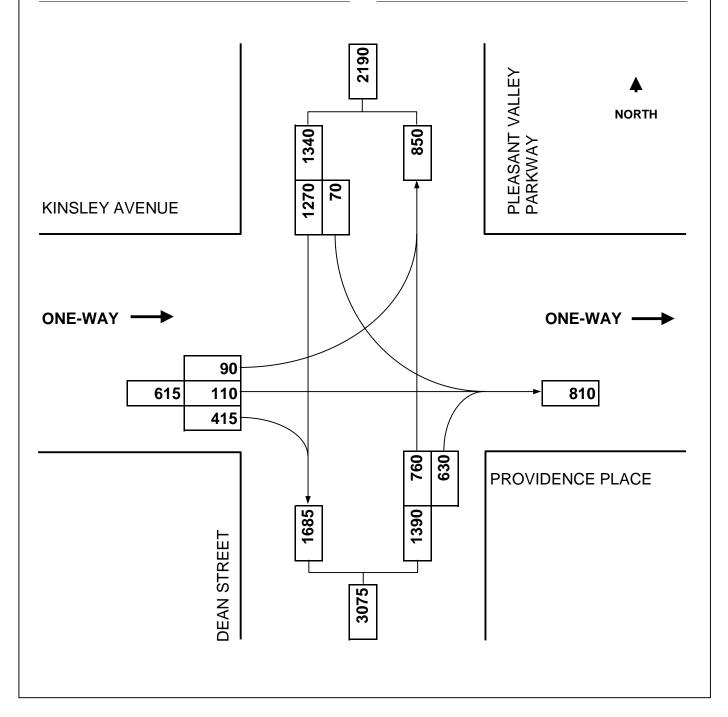
Reference No.: 5999

Existing: n/a

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: AM Peak Hour



	•	-	•	•	•	•	4	†	/	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7					↑ ↑		7	^	
Traffic Volume (vph)	90	110	415	0	0	0	0	760	630	70	1270	0
Future Volume (vph)	90	110	415	0	0	0	0	760	630	70	1270	0
Satd. Flow (prot)	0	1858	1583	0	0	0	0	3328	0	1805	3539	0
Flt Permitted		0.978								0.118		
Satd. Flow (perm)	0	1858	1583	0	0	0	0	3328	0	224	3539	0
Satd. Flow (RTOR)			85					267				
Lane Group Flow (vph)	0	209	432	0	0	0	0	1433	0	71	1296	0
Turn Type	Split	NA	Perm					NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2							1		
Total Split (s)	27.0	27.0	27.0					39.0		24.0		
Total Lost Time (s)		5.0	5.0					5.0		5.0		
Act Effct Green (s)		22.0	22.0					34.0		53.0	58.0	
Actuated g/C Ratio		0.24	0.24					0.38		0.59	0.64	
v/c Ratio		0.46	0.96					1.01		0.15	0.57	
Control Delay		32.9	62.1					49.5		7.1	2.4	
Queue Delay		0.0	0.0					0.0		1.0	8.0	
Total Delay		32.9	62.1					49.5		8.1	3.2	
LOS		С	Е					D		Α	Α	
Approach Delay		52.6						49.5			3.5	
Approach LOS		D						D			Α	
Queue Length 50th (ft)		101	201					~370		2	15	
Queue Length 95th (ft)		169	#391					#534		m5	m45	
Internal Link Dist (ft)		115			721			165			84	
Turn Bay Length (ft)			50							100		
Base Capacity (vph)		454	451					1423		465	2281	
Starvation Cap Reductn		0	0					0		246	622	
Spillback Cap Reductn		0	0					0		0	0	
Storage Cap Reductn		0	0					0		0	0	
Reduced v/c Ratio		0.46	0.96					1.01		0.32	0.78	

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 31.8

Intersection LOS: C ICU Level of Service H

Intersection Capacity Utilization 126.1%

Analysis Period (min) 15

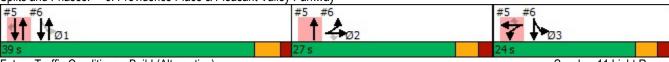
 Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Providence Place & Pleasant Valley Parkway



Future Traffic Conditions - Build (Alternative)

Timing Plan: AM Peak Hour



Major Street: Dean Street

City/Town: Providence, RI

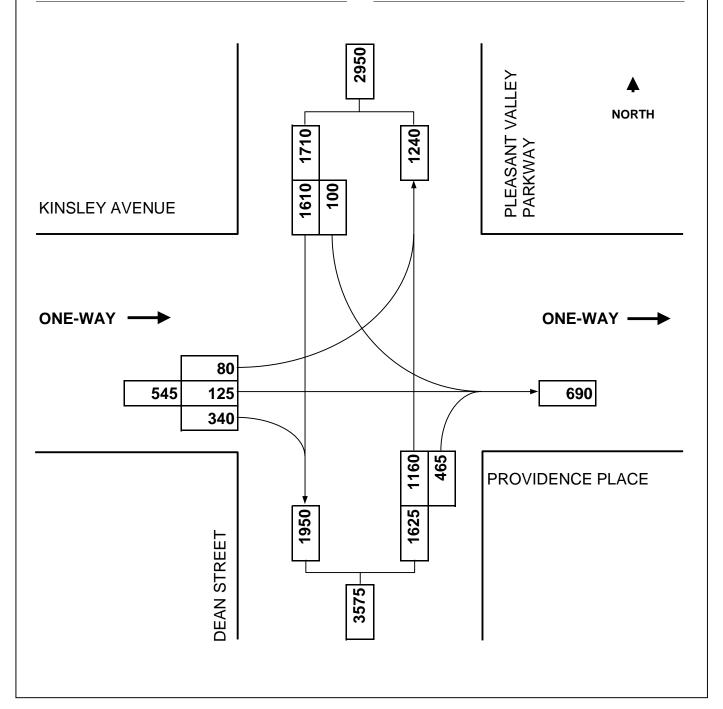
Reference No.: 5999

Existing: n/a

Minor Street: Kinsley Avenue

Day of Week: Weekday

Peak Period: PM Peak Hour



	•	-	•	•	•	•	4	†	-	-	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स	7					↑ ↑		*	^	
Traffic Volume (vph)	80	125	340	0	0	0	0	1160	465	100	1610	0
Future Volume (vph)	80	125	340	0	0	0	0	1160	465	100	1610	0
Satd. Flow (prot)	0	1864	1599	0	0	0	0	3406	0	1805	3539	0
Flt Permitted		0.981								0.103		
Satd. Flow (perm)	0	1864	1599	0	0	0	0	3406	0	196	3539	0
Satd. Flow (RTOR)			85					84				
Lane Group Flow (vph)	0	213	354	0	0	0	0	1675	0	102	1643	0
Turn Type	Split	NA	Perm					NA		D.P+P	NA	
Protected Phases	2	2						1		3	13	
Permitted Phases			2							1		
Total Split (s)	18.0	18.0	18.0					44.0		28.0		
Total Lost Time (s)		5.0	5.0					5.0		5.0		
Act Effct Green (s)		13.0	13.0					39.0		62.0	67.0	
Actuated g/C Ratio		0.14	0.14					0.43		0.69	0.74	
v/c Ratio		0.79	1.17					1.10		0.19	0.62	
Control Delay		59.5	134.0					80.7		5.4	1.1	
Queue Delay		0.0	0.0					0.0		1.5	0.7	
Total Delay		59.5	134.0					80.7		6.9	1.9	
LOS		Е	F					F		Α	Α	
Approach Delay		106.0						80.7			2.2	
Approach LOS		F						F			Α	
Queue Length 50th (ft)		119	~198					~561		2	13	
Queue Length 95th (ft)		#232	#370					#700		m2	m14	
Internal Link Dist (ft)		115			721			165			84	
Turn Bay Length (ft)			50							100		
Base Capacity (vph)		269	303					1523		546	2634	
Starvation Cap Reductn		0	0					0		311	596	
Spillback Cap Reductn		0	0					0		0	0	
Storage Cap Reductn		0	0					0		0	0	
Reduced v/c Ratio		0.79	1.17					1.10		0.43	0.81	

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.17

Intersection Signal Delay: 49.9 Intersection Capacity Utilization 132.8% Intersection LOS: D

ICU Level of Service H

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

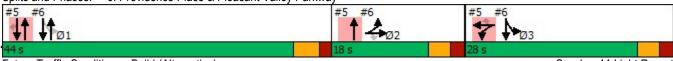
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Providence Place & Pleasant Valley Parkway



Future Traffic Conditions - Build (Alternative)

Timing Plan: PM Peak Hour

Pleasant Valley Parkway at Promenade Street





Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

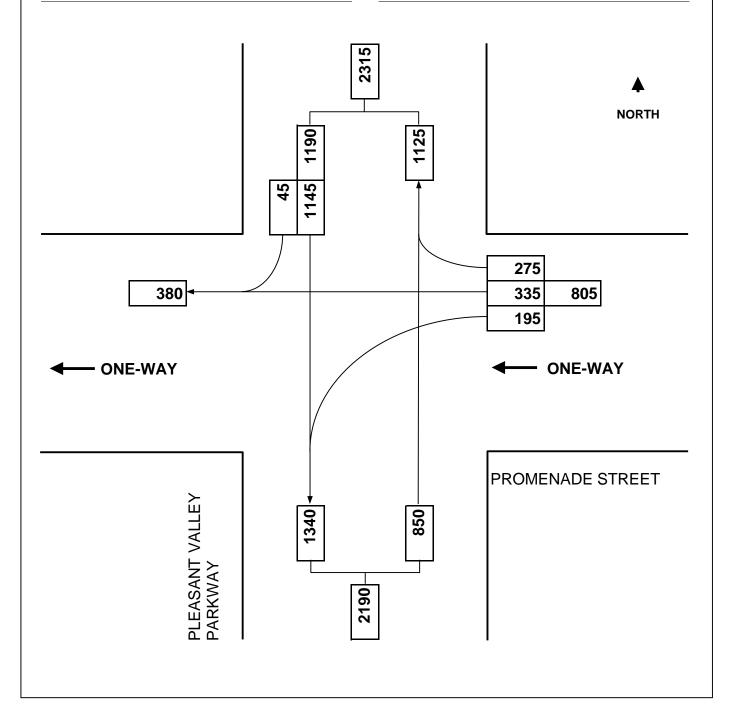
Reference No.: 5999

Existing: n/a

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: AM Peak Hour



	۶	→	•	•	•	•	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				1	ર્ન	7		^			^	7
Traffic Volume (vph)	0	0	0	195	335	275	0	850	0	0	1145	45
Future Volume (vph)	0	0	0	195	335	275	0	850	0	0	1145	45
Satd. Flow (prot)	0	0	0	1665	1780	1583	0	3610	0	0	3610	1615
Flt Permitted				0.950	0.997							
Satd. Flow (perm)	0	0	0	1665	1780	1583	0	3610	0	0	3610	1615
Satd. Flow (RTOR)						208						85
Lane Group Flow (vph)	0	0	0	181	365	284	0	859	0	0	1180	46
Turn Type				Split	NA	Perm		NA			NA	Perm
Protected Phases				3	3			12			1	
Permitted Phases						3						1
Total Split (s)				24.0	24.0	24.0					39.0	39.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				19.0	19.0	19.0		61.0			34.0	34.0
Actuated g/C Ratio				0.21	0.21	0.21		0.68			0.38	0.38
v/c Ratio				0.52	0.97	0.57		0.35			0.87	0.07
Control Delay				37.5	77.5	14.5		1.1			34.1	1.4
Queue Delay				0.0	0.0	0.0		2.0			0.0	0.0
Total Delay				37.5	77.5	14.5		3.1			34.1	1.4
LOS				D	Е	В		Α			С	Α
Approach Delay					47.2			3.1			32.9	
Approach LOS					D			Α			С	
Queue Length 50th (ft)				96	218	36		14			320	0
Queue Length 95th (ft)				166	#403	114		m14			#419	7
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				351	375	498		2447			1363	663
Starvation Cap Reductn				0	0	0		1386			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				0.52	0.97	0.57		0.81			0.87	0.07

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 28.2
Intersection Capacity Utilization 126.1%

Intersection LOS: C
ICU Level of Service H

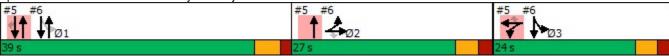
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street



Lane Group	Ø2
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Satd. Flow (RTOR)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	2
Permitted Phases	
Total Split (s)	27.0
Total Lost Time (s)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	
intoroccion cummary	



Major Street: Pleasant Valley Pkwy.

City/Town: Providence, RI

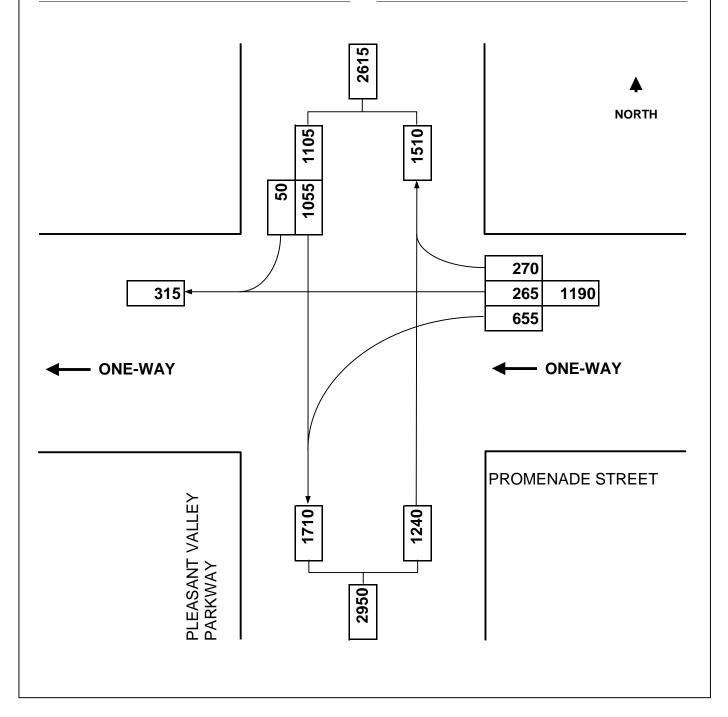
Reference No.: 5999

Existing: n/a

Minor Street: Promenade Street

Day of Week: Weekday

Peak Period: PM Peak Hour



	۶	→	•	•	•	•	1	†	-	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				1	ર્ન	7		^			^	7
Traffic Volume (vph)	0	0	0	655	265	270	0	1240	0	0	1055	50
Future Volume (vph)	0	0	0	655	265	270	0	1240	0	0	1055	50
Satd. Flow (prot)	0	0	0	1665	1735	1583	0	3610	0	0	3610	1615
Flt Permitted				0.950	0.979							
Satd. Flow (perm)	0	0	0	1665	1735	1583	0	3610	0	0	3610	1615
Satd. Flow (RTOR)						85						85
Lane Group Flow (vph)	0	0	0	466	482	278	0	1253	0	0	1088	52
Turn Type				Split	NA	Perm		NA			NA	Perm
Protected Phases				3	3			12			1	
Permitted Phases						3						1
Total Split (s)				28.0	28.0	28.0					44.0	44.0
Total Lost Time (s)				5.0	5.0	5.0					5.0	5.0
Act Effct Green (s)				23.0	23.0	23.0		57.0			39.0	39.0
Actuated g/C Ratio				0.26	0.26	0.26		0.63			0.43	0.43
v/c Ratio				1.10	1.09	0.60		0.55			0.70	0.07
Control Delay				106.2	102.6	26.1		0.7			23.6	1.6
Queue Delay				0.0	0.0	0.0		7.4			0.0	0.0
Total Delay				106.2	102.6	26.1		8.1			23.6	1.6
LOS				F	F	С		Α			С	Α
Approach Delay					86.6			8.1			22.6	
Approach LOS					F			Α			С	
Queue Length 50th (ft)				~318	~327	95		8			256	0
Queue Length 95th (ft)				#514	#525	178		m13			329	9
Internal Link Dist (ft)		491			792			84			494	
Turn Bay Length (ft)						150						
Base Capacity (vph)				425	443	467		2286			1564	748
Starvation Cap Reductn				0	0	0		987			0	0
Spillback Cap Reductn				0	0	0		0			0	0
Storage Cap Reductn				0	0	0		0			0	0
Reduced v/c Ratio				1.10	1.09	0.60		0.96			0.70	0.07

Cycle Length: 90

Actuated Cycle Length: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.17

Intersection Signal Delay: 39.3 Intersection Capacity Utilization 132.8% Intersection LOS: D ICU Level of Service H

Analysis Period (min) 15

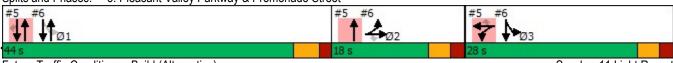
 Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Pleasant Valley Parkway & Promenade Street



Future Traffic Conditions - Build (Alternative)

Timing Plan: PM Peak Hour

Lane Group	Ø2		
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Satd. Flow (RTOR)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	2		
Permitted Phases			
Total Split (s)	18.0		
Total Lost Time (s)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (ft)			
Queue Length 95th (ft)			
Internal Link Dist (ft)			
Turn Bay Length (ft)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			
intoroccion cuminary			

Kinsley Avenue at Acorn Street





Major Street: Kinsley Street

City/Town: Providence, RI

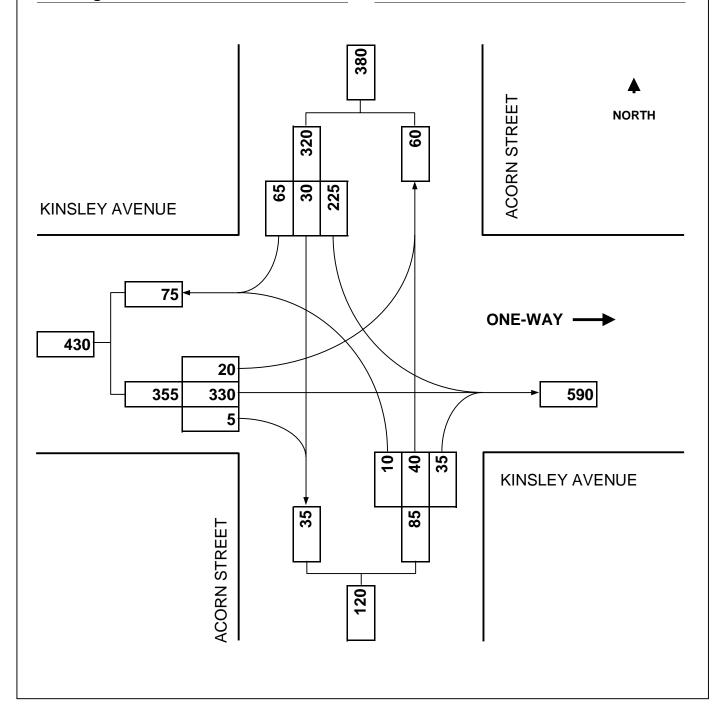
Reference No.: 5999

Existing: n/a

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: AM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Vol, veh/h	20	330	5	0	0	0	10	40	35	225	30	65
Future Vol, veh/h	20	330	5	0	0	0	10	40	35	225	30	65
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	2	0	0	0	0	0	2	0	0	0	0
Mvmt Flow	22	359	5	0	0	0	11	43	38	245	33	71
Number of Lanes	0	1	0	0	0	0	0	1	0	0	1	0
Approach	EB						NB			SB		
Opposing Approach							SB			NB		
Opposing Lanes	0						1			1		
Conflicting Approach Left	SB						EB					
Conflicting Lanes Left	1						1			0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	1						0			1		
HCM Control Delay	13.7						9.1			12.9		
HCM LOS	В						Α			В		

Lane	NBLn1	EBLn1	SBLn1	
Vol Left, %	12%	6%	70%	
Vol Thru, %	47%	93%	9%	
Vol Right, %	41%	1%	20%	
Sign Control	Stop	Stop	Stop	
Traffic Vol by Lane	85	355	320	
LT Vol	10	20	225	
Through Vol	40	330	30	
RT Vol	35	5	65	
Lane Flow Rate	92	386	348	
Geometry Grp	1	1	1	
Degree of Util (X)	0.135	0.534	0.486	
Departure Headway (Hd)	5.241	4.981	5.026	
Convergence, Y/N	Yes	Yes	Yes	
Сар	688	714	710	
Service Time	3.241	3.071	3.115	
HCM Lane V/C Ratio	0.134	0.541	0.49	
HCM Control Delay	9.1	13.7	12.9	
HCM Lane LOS	А	В	В	
HCM 95th-tile Q	0.5	3.2	2.7	



Major Street: Kinsley Street

City/Town: Providence, RI

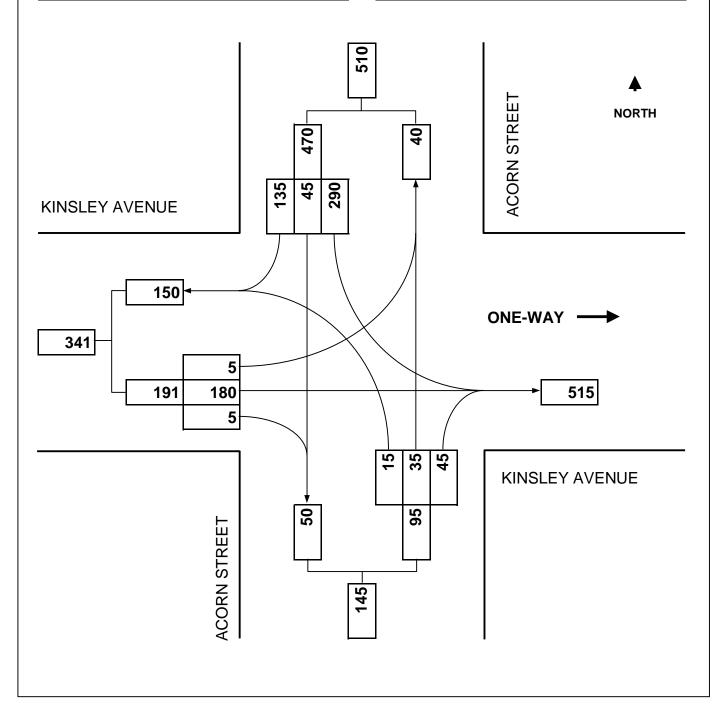
Reference No.: 5999

Existing: n/a

Minor Street: Acorn Street

Day of Week: Weekday

Peak Period: PM Peak Hour



ntersection	
ntersection Delay, s/veh	13.4
ntersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Vol, veh/h	5	180	5	0	0	0	15	35	45	290	45	135
Future Vol, veh/h	5	180	5	0	0	0	15	35	45	290	45	135
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	2	0	0	2	0	0	0	0	0	0	0
Mvmt Flow	5	196	5	0	0	0	16	38	49	315	49	147
Number of Lanes	0	1	0	0	0	0	0	1	0	0	1	0
Approach	EB						NB			SB		
Opposing Approach							SB			NB		
Opposing Lanes	0						1			1		
Conflicting Approach Left	SB						EB					
Conflicting Lanes Left	1						1			0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	1						0			1		
HCM Control Delay	10.7						8.6			15.4		
HCM LOS	В						Α			С		

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	16%	3%	62%
Vol Thru, %	37%	95%	10%
Vol Right, %	47%	3%	29%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	95	190	470
LT Vol	15	5	290
Through Vol	35	180	45
RT Vol	45	5	135
Lane Flow Rate	103	207	511
Geometry Grp	1	1	1
Degree of Util (X)	0.137	0.303	0.643
Departure Headway (Hd)	4.784	5.276	4.53
Convergence, Y/N	Yes	Yes	Yes
Сар	742	675	792
Service Time	2.86	3.353	2.58
HCM Lane V/C Ratio	0.139	0.307	0.645
HCM Control Delay	8.6	10.7	15.4
HCM Lane LOS	Α	В	С
HCM 95th-tile Q	0.5	1.3	4.8

Dean Street at Site Driveway





Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

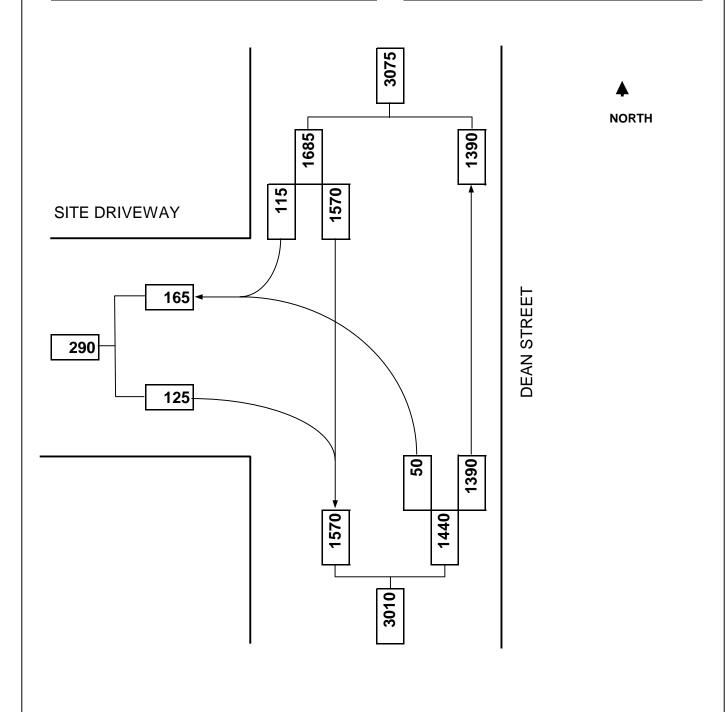
Existing: n/a

Minor Street: Site Driveway

Day of Week: Weekday

Peak Period: AM Peak Hour

Future: 2023 Build Alternative



land a second disconnection of						
Intersection	1 4					
Int Delay, s/veh	1.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7	ħ	^	†	
Traffic Vol, veh/h	0	125	50	1390	1570	115
Future Vol, veh/h	0	125	50	1390	1570	115
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Stop	-	None	-	None
Storage Length	-	-	50	-	-	-
Veh in Median Storage,	# 0	-	_	0	0	-
Grade, %	0	-	_	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	0
Mymt Flow	0	136	54	1511	1707	125
WWW	v	100	01	1011	1101	120
Major/Minor Mi	inor2	N	Major1	N	//ajor2	
Conflicting Flow All	-	916	1832	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	_	-	_	-
Follow-up Hdwy	_	3.3	2.2	-	_	-
Pot Cap-1 Maneuver	0	279	338	-	-	_
Stage 1	0		-	_	_	_
Stage 2	0	_	_	_	_	_
Platoon blocked, %	U			_	_	_
Mov Cap-1 Maneuver	_	279	338			
Mov Cap-1 Maneuver	_	219	330	-	-	
		-	-	_	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
	29.6		0.6		0	
HCM LOS	D					
	_					
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		338	-		-	-
HCM Lane V/C Ratio		0.161	-	0.487	-	-
HCM Control Delay (s)		17.7	-	29.6	-	-
HCM Lane LOS		С	-	D	-	-
HCM 95th %tile Q(veh)		0.6	-	2.5	-	-
., ,						



Major Street: Dean Street

City/Town: Providence, RI

Reference No.: 5999

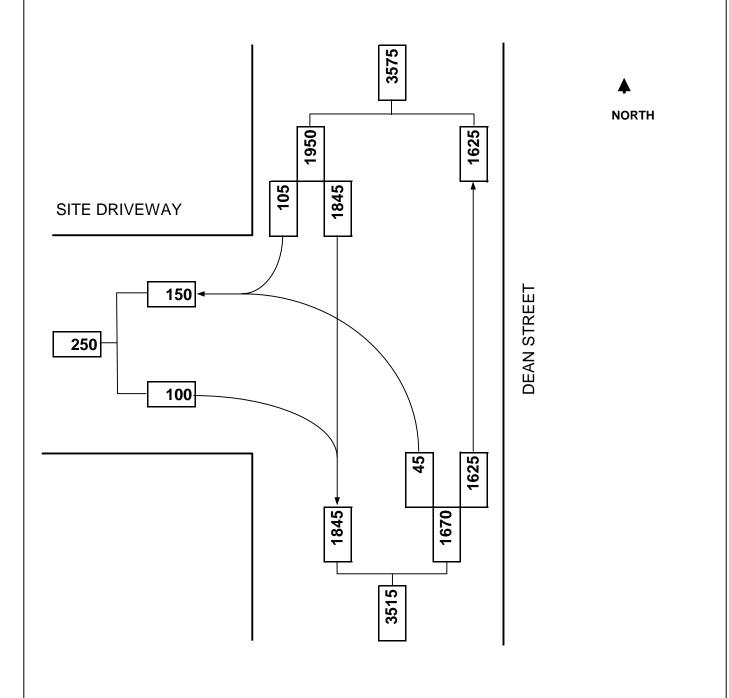
Existing: n/a

Minor Street: Site Driveway

Day of Week: Weekday

Peak Period: PM Peak Hour

Future: 2023 Build Alternative



Intersection						
Int Delay, s/veh	1.2					
		EDD	ND	NDT	ODT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7	7	^	†	
Traffic Vol, veh/h	0	100	45	1625	1845	105
Future Vol, veh/h	0	100	45	1625	1845	105
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Stop	-	None	-	None
Storage Length	-	-	50	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	109	49	1766	2005	114
	-					
				_		
	linor2		Major1		Major2	
Conflicting Flow All	-	1060	2119	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	_	-	-	-	_	-
Follow-up Hdwy	_	3.3	2.2	_	_	-
Pot Cap-1 Maneuver	0	224	261	_	-	_
Stage 1	0			_	_	_
Stage 2	0	_	_	_	_	_
Platoon blocked, %	U			_	_	_
Mov Cap-1 Maneuver	_	224	261	_	_	_
			201			
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	35.3		0.6		0	
HCM LOS	55.5 E		3.0			
1 TOWN EOO						
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		261	-	224	-	-
HCM Lane V/C Ratio		0.187	-	0.485	-	-
HCM Control Delay (s)		21.9	-	35.3	-	-
HCM Lane LOS		C	_	E	_	-
HCM 95th %tile Q(veh)		0.7	_	2.4	_	-
		V.1				

Kinsley Avenue at Western Site Driveway





Major Street: Kinsley Avenue

City/Town: Providence, RI

Reference No.: 5999

n/a

Existing:

Minor Street: Western Site Driveway

Day of Week: Weekday

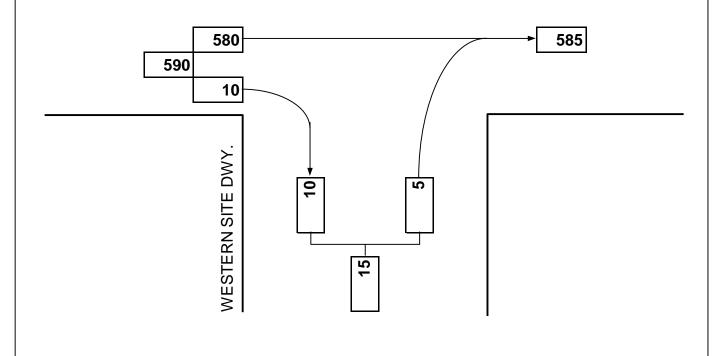
Peak Period: AM Peak Hour

Future: 2023 Build Alternative



NORTH

KINSLEY AVENUE



Intersection						
Int Delay, s/veh	0.1					
		EDD	MD	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽					7
Traffic Vol, veh/h	580	10	0	0	0	5
Future Vol, veh/h	580	10	0	0	0	5
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	-	-	-	-	-	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	0	0	0
Mvmt Flow	630	11	0	0	0	5
NA ' (NA'						
	lajor1			<u> </u>	/linor1	
Conflicting Flow All	0	0			-	636
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Critical Hdwy	-	-			-	6.2
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	-	-			-	-
Follow-up Hdwy	-	-			-	3.3
Pot Cap-1 Maneuver	-	-			0	481
Stage 1	_	-			0	-
Stage 2	-	-			0	-
Platoon blocked, %	_	_				
Mov Cap-1 Maneuver	_	_			_	481
Mov Cap-1 Maneuver	_	_			_	01
Stage 1	_				-	
Stage 2	_				_	-
Staye 2	-	-			-	-
Approach	EB				NB	
HCM Control Delay, s	0				12.6	
HCM LOS					В	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR		
Capacity (veh/h)		481	-	-		
HCM Lane V/C Ratio		0.011	-	-		
HCM Control Delay (s)		12.6	-	-		
HCM Lane LOS		В	-	-		
HCM 95th %tile Q(veh)		0	-	-		



Major Street: Kinsley Avenue

City/Town: Providence, RI

Reference No.: 5999

Existing: n/a

Minor Street: Western Site Driveway

Day of Week: Weekday

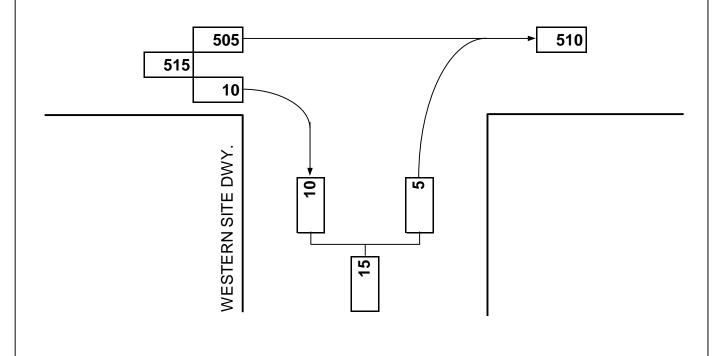
Peak Period: PM Peak Hour

Future: 2023 Build Alternative



NORTH

KINSLEY AVENUE



Intersection	0.1					
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1→					7
Traffic Vol, veh/h	505	10	0	0	0	5
Future Vol, veh/h	505	10	0	0	0	5
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	_	-	_	0
Veh in Median Storage, #		_	-	0	0	
						-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	0	0	0
Mvmt Flow	549	11	0	0	0	5
Major/Minor Ma	ajor1			N	/linor1	
Conflicting Flow All	0	0			-	555
		-				555
Stage 1	-				-	-
Stage 2	-	-			-	-
Critical Hdwy	-	-			-	6.2
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	-	-			-	-
Follow-up Hdwy	-	-			-	3.3
Pot Cap-1 Maneuver	-	-			0	535
Stage 1	-	-			0	-
Stage 2	-	-			0	-
Platoon blocked, %	-	-				
Mov Cap-1 Maneuver	-	_			-	535
Mov Cap-2 Maneuver	_	_			_	-
Stage 1	_	_			_	_
Stage 2	_	_			_	_
Staye 2		_				_
Approach	EB				NB	
HCM Control Delay, s	0				11.8	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR		
Capacity (veh/h)		535	-	-		
HCM Lane V/C Ratio		0.01	-	-		
HCM Control Delay (s)		11.8	-	-		
HCM Lane LOS		В	-	-		
HCM 95th %tile Q(veh)		0	-	_		

Kinsley Avenue at Eastern Site Driveway





Major Street: Kinsley Avenue City/Town: Providence, RI Reference No.: 5999

Existing: n/a Minor Street: Eastern Site Driveway

Day of Week: Weekday

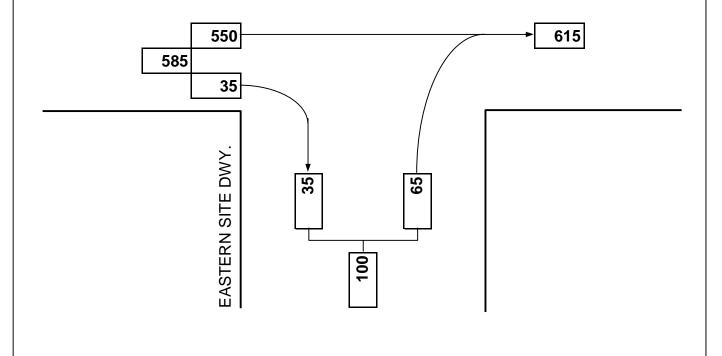
Peak Period: AM Peak Hour

Future: 2023 Build Alternative



NORTH

KINSLEY AVENUE



Intersection						
Int Delay, s/veh	1.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1					7
Traffic Vol, veh/h	550	35	0	0	0	65
Future Vol, veh/h	550	35	0	0	0	65
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized		None				None
	-		-		-	
Storage Length	-	-	-	-	-	0
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	0	0	0
Mvmt Flow	598	38	0	0	0	71
Major/Minor M	oior1				linar1	
	ajor1			IN.	/linor1	0.17
Conflicting Flow All	0	0			-	617
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Critical Hdwy	-	-			-	6.2
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	-	-			-	-
Follow-up Hdwy	-	-			-	3.3
Pot Cap-1 Maneuver	_	-			0	494
Stage 1	_	_			0	_
Stage 2	_	_			0	_
Platoon blocked, %	_	_			U	
						494
Mov Cap-1 Maneuver	-				-	
Mov Cap-2 Maneuver	-	-			-	-
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Approach	EB				NB	
HCM Control Delay, s	0				13.5	
	U				13.5 B	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR		
Capacity (veh/h)		494	-	_		
HCM Lane V/C Ratio		0.143	_	_		
HCM Control Delay (s)		13.5		_		
HCM Lane LOS		13.5 B	_	_		
			-	-		
HCM 95th %tile Q(veh)		0.5	-	-		



Major Street: Kinsley Avenue

City/Town: Providence, RI

Reference No.: 5999

n/a

Existing:

Minor Street: Eastern Site Driveway

Day of Week: Weekday

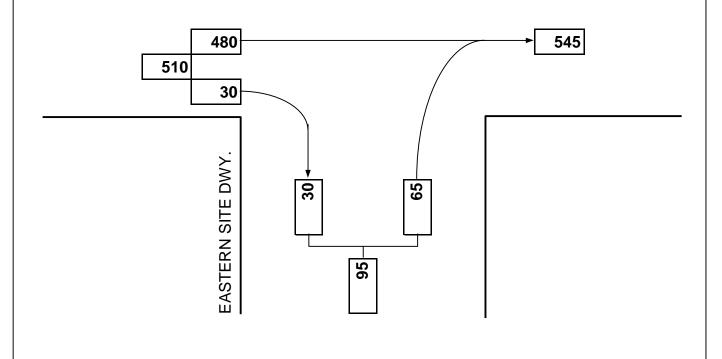
Peak Period: PM Peak Hour

Future: 2023 Build Alternative



NORTH

KINSLEY AVENUE



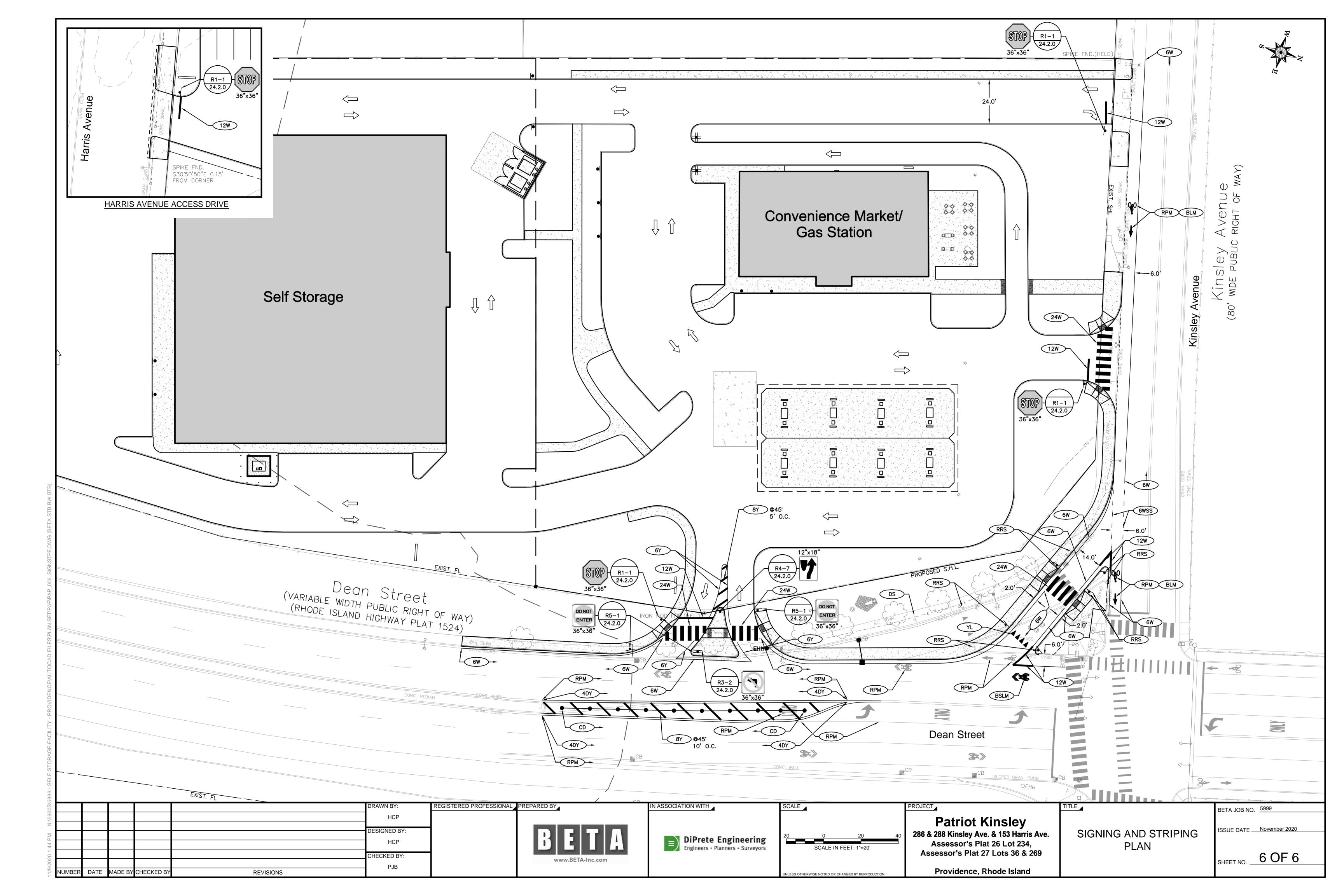
Intersection						
Int Delay, s/veh	1.4					
		EDD	WDL	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	100	00	^	^	^	7
Traffic Vol, veh/h	480	30	0	0	0	65
Future Vol, veh/h	480	30	0	0	0	65
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storag	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	0	0	0
Mvmt Flow	522	33	0	0	0	71
NA : (NA:					P 4	
Major/Minor	Major1			N	/linor1	
Conflicting Flow All	0	0			-	539
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Critical Hdwy	-	-			-	6.2
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	-	-			-	-
Follow-up Hdwy	-	-			-	3.3
Pot Cap-1 Maneuver	-	-			0	546
Stage 1	-	-			0	-
Stage 2	_	_			0	_
Platoon blocked, %	_	_				
Mov Cap-1 Maneuver		_			_	546
Mov Cap-2 Maneuver		_			_	-
Stage 1	_				_	_
		-				
Stage 2	-	-			-	-
Approach	EB				NB	
HCM Control Delay, s	. 0				12.6	
HCM LOS					В	
					_	
Minor Lane/Major Mvi	mt I	NBLn1	EBT	EBR		
Capacity (veh/h)		546	-	-		
HCM Lane V/C Ratio		0.129	-	-		
HCM Control Delay (s	s)	12.6	-	-		
HCM Lane LOS		В	-	-		
HCM 95th %tile Q(veh	ո)	0.4	-	-		
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APPENDIX E – Off-Site Improvement Plan

Dean Street at Kinsley Avenue/Providence Place





APPENDIX F – Build Alternative Plan (Woonasquatucket River Greenway)

Dean Street at Kinsley Avenue/Providence Place



